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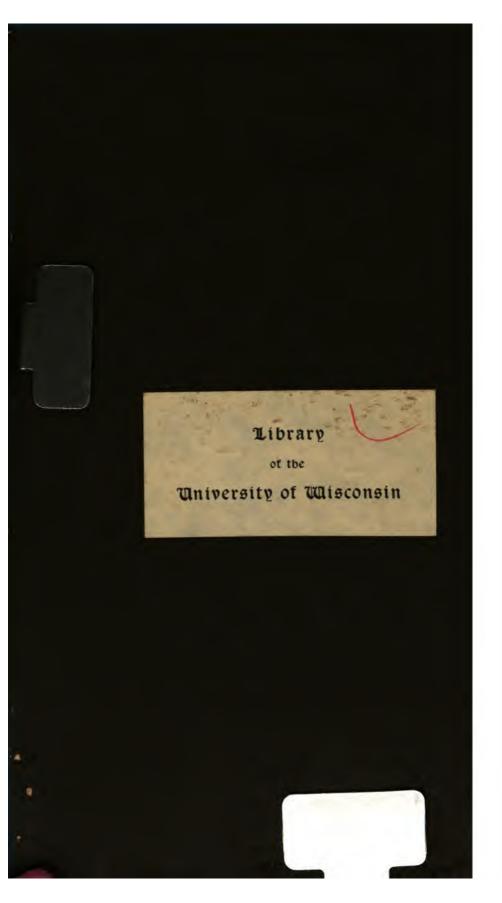
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HYDRAULIC TABLES

SHOWING THE LOSS OF HEAD DUE

TO THE FRICTION OF WATER FLOWING IN

PIPES, AQUEDUCTS, SEWERS, ETC.

AND

THE DISCHARGE OVER WEIRS

BY

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CONTENTS.

9	AGE
Introduction	1
Formula Used	1
INCREASING FRICTION WITH AGE, HOW COMPUTED, AND INDICATED IN THE	
Tables	2
Observations of Flow in Cast-iron Pipe	4
" " " RIVETED STEEL PIPE	5
" " WOODEN-STAVE PIPE	5
" " RECTANGULAR WOODEN PIPE	6
" CEMENT PIPE	6
" " " WROUGHT-IRON PIPE	6
" GALVANIZED-IRON PIPE	6
" Brass Pipe	7
" " LEAD PIPE	7
" GLASS PIPE.	7
" " FIRE-HOSE	7
" " " OPEN CONDUITS	9
" " AQUEDUCTS	11
" " " BRICK SEWERS.	11
" " CANALS	11
TABLE OF FLOW IN SMALL BRASS PIPES.	15
" " " WROUGHT-IRON PIPES	
" " " Hose and Pipes	
" " " PIPES, 4" TO 144" DIAMETER	
Aqueducts, Relative Discharging Capacity of.	
" Table of Flow in	
SEWERS, TABLE OF SLOPES REQUIRED TO PRODUCE CERTAIN VELOCITIES	-51 52
TILE SEWER TABLE	
CIRCULAR BRICKSEWER TABLE	
DECREASE IN CARRYING CAPACITY OF CAST-IRON PIPE WITH AGE	-51 58
Comparison of Results with those of Coppin and Weston	59
METRIC PIPE TABLE.	60
VENTURI METER, LOSS OF HEAD IN	61
Underdrains, for Sand-filters.	62
Underdrains, for Sand-Filters. Flow over Weirs.	
FLOW UTER TEERS	ua

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INTRODUCTION.

THE following tables show the flow of water in pipes and other passages, as computed by the Hazen-Williams hydraulic slide-rule, based upon the formula

$$v = cr^{0.63}s^{0.54}0.001^{-0.04}$$
.

The most commonly used formula for determining the velocity of flow of water in pipes and channels is the Chezy formula, namely,

$$v = c\sqrt{sr}$$

where v is the velocity in feet per second, s is the hydraulic slope, and r the hydraulic radius in feet. c is a factor the value of which is an approximation to a constant, but depends upon the roughness of the pipe and upon the hydraulic radius and slope. The variations in the value of c are considerable, and make the general use of the formula difficult.

Kutter's formula was devised to compute the value of c in the Chezy formula. The value of c so computed depends upon an assumed coefficient of roughness, upon the slope, and upon the hydraulic radius. With the same degree of roughness the value of c increases with the hydraulic slope and with the hydraulic radius. This is because the exponents used for these terms in the formula are below the true values. If the exponents were increased to correspond more nearly with the facts, the variations in the value of c would become less. If exponents could be selected agreeing perfectly with the facts, the value of c would depend upon the roughness only, and for any given degree of roughness c would then be a constant. It is not possible to reach this actually, because the values of the exponents vary with different surfaces, and also their values may not be exactly the same for large diameters and for small ones, nor for steep slopes and for flat ones. Exponents can be selected, however, representing approximately average conditions, so that the value of cfor a given condition of surface will vary so little as to be practically constant. Several such "exponential" formulas have been suggested. These formulas are among the most satisfactory yet devised, but their use has been limited by the difficulty in making computations by them.

This difficulty was eliminated by the use of a slide-rule constructed for that purpose.

The exponents in the formula used were selected as representing as nearly as possible average conditions, as deduced from the best available records of experiments upon the flow of water in such pipes and channels as most frequently occur in water-works practice. The last term, $0.001^{-0.04}$, is a constant, and is introduced simply to equalize the value of c with the value in the Chezy formula, and other exponential formulas which may be used, at a slope of 0.001 instead of at a slope of 1.

The slide-rules were furnished by Mr. G. G. Ledder, 9 Province Court, Boston, Mass., the work being done in Germany. Suitable scales were laid out and the position of each graduation was computed to 0.01 millimeter. The drawings were then engraved upon steel and reproduced upon slide-rules of the general size and appearance of the ordinary 10-inch Mannheim rule. The graduation is very perfectly done, and the accuracy obtained is practically that which can be secured with the ordinary slide-rule of this size.

All the computations of figures contained in this volume, except a few fundamental ratios, have been made with the slide-rule, and only such accuracy has been sought as can readily be obtained by this method of computation.

This formula has been used by the authors for some time, and it is hoped that the tables will be useful to those not accustomed to the use of the slide-rule, and also to those who use the slide-rule, as a reference showing velocities and velocity heads, and establishing beyond question the position of the decimal point, which is the most troublesome feature in the use of the slide-rule to beginners.

These tables are not confined to a single value of the coefficient of roughness, which is called c. Instead, a series of values of c is given in the various columns, and under each are placed the corresponding losses of head. The headings also indicate in a general way the class of pipe for which the particular coefficient should be used, but these indications are only general, and it is the intention to leave the matter so that users can select such values of c as in their judgment represent the particular conditions upon which they are figuring.

The gradual roughening of the interior of cast-iron pipe is one of the most familiar of water-works phenomena. It is also one of the most difficult to compute. In a general way it may be said that in a series of years, which is not long compared with the total life of the pipe, the roughening of the surface and the reduction of the area through rusting and tuberculation reach such an extent that twice as much head is consumed in sending a given volume of water through it as was the case when the pipe was new.

In a particular set of foreign tables, based on the Darcy formula,

the loss of head is given for new pipe, and in the second column, designated old pipe, a figure twice as large is given. This has certain advantages over a table of factors to be applied to pipes of different ages, as has been done in several American publications, because it is less apt to be forgotten; and while it is a crude precedure, it keeps in mind the fact that old pipe will pass very much less water than new pipe.

In this volume effort has been made to put this subject in better shape. It is a difficult matter to handle adequately, for no two pieces of iron pipe deteriorate at the same rate, and any figures given are therefore at best only approximations to averages, which averages may be very far from individual cases.

The system used is to put certain figures surrounded by circles over the columns. This mark was adopted because no words could be found sufficiently concise and at the same time accurate. Over the column for c = 140 are placed two zeros in a circle: (00). That indicates that this coefficient is obtained with the very best cast-iron pipe, laid perfectly straight, and when new. Over c=130 is placed one zero in a circle, (0), and this is the value that can be fairly counted on for good new castiron pipe. Over the following columns are placed figures in circles. These figures show the age in years at which, on an average, as nearly as we know, cast-iron pipe will reach the values given in the column underneath. It must be understood that these are necessarily very rough approximations, based on the best data available, which are principally for soft and clear but unfiltered river-waters. Hard waters and lake waters will often attack the pipe less rapidly, and the figures must then be increased. Sometimes they must be multiplied by two or more. Other waters will corrode the pipes more rapidly than the average, and for them the values will be reached more quickly than the figures indicate.

The divergence with different castings and with different kinds of water is greatest in the smallest pipes, and no attempt is made to extend the figures in the circles to the sizes below four inches in diameter.

Steel pipes tuberculate and corrode in much the same manner as cast-iron pipes. On account of the rivets and in-and-out joints the average value of c is lower than for cast-iron pipe. The data at hand indicate a value of 110 for new pipe, decreasing in the course of about ten years to 100. For older pipes, as far as the present data go, steel pipe of a given age will carry the same quantity of water as cast-iron pipe of the same size and ten years older.

On the Value of c.—In the Engineering Record of March 28, 1903, was published by the authors a table of the values of c computed from published experiments upon the friction of water in pipes and conduits of various kinds, the results being selected as the most reliable available data. This table, with some additions, is as follows:

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c in Wean Remarks.		(i. 125.8 129	34 132 Paris main 14 119 "." 86 93 Boston main 03 107 Paris main 06 107 "." 41 142 Rosemary siphon
Range of c in H. & W. Formula	N PIPE.	119.5 to 120.0 132.1 " 125.8 132.1 " 125.8 125.0 " 116.0 139.3 " 148.5 107.0 " 121.5 106.0 " 145.6 145.0 " 145.6 145.0 " 145.6 129.0 " 147.8 112.0 " 117.8 129 142.0 to 141.0 142.0 to 141.0	130 to 134 124 ** 114 100 ** 86 110 ** 103 107 ** 106 144 ** 141
Range of Velocity, Feet per Second.	NEW CAST-IRON PIPE.	0.36 to 5.15 0.5 '' 7.48 132.1 1.6 '' 8.22 1.25.0 1.0 '' 5.00 1.6 '' 3.1 1.6 '' 3.1 1.6 '' 3.1 1.6 '' 3.1 1.6 '' 3.1 1.6 '' 3.1 1.20 1.4 '' 3.1 1.25 '' 2.90 1.35 2.6 to 6.2 1.42.0 0.73 to 1.10 CLEANED CAST-IRON PIPE.	0.4 to 3.7 0.6 " 5.0 0.95 " 2.46 0.9 " 8.44 0.8 " 10.4 2.0 " 5.0
Num- ber of Obser- vations.		889844684086	
Diameter in Inches.		3.22 2.3.39 1.2.1.2 1.6.02 1.6.02 2.9.9.68 8.88 8.89 8.89 8.89 8.89 8.89 8.	1.43 3.16 6 9.63 11.69
Experimenter.		Darcy Williams, Hubbell, Fenkell liben Williams, Hubbell, Ferkell lismpe Lampe Darcy Williams, Hubbell, Ferkell Kuichling Stearns Gale Fenkell	Darcy. Brackett. Darcy. FitzGerald.

	Paris main Boston main '' '' '' '' '' '' '' '' ''		Coated sheet iron Galvanized sheet iron Coated sheet iron	Riveted sheet iron Coated sheet iron Riveted sheet iron	Riveted steel Riveted sheet steel Riveted steel	Riveted sheet steel		Los Angeles Seattle Ógden
	83 116 118 34 43 84 106	- -	128	123	000000000000000000000000000000000000000	97 111 114	_	123 123 120 120
IRON PIPE.	86 to 76 82 " 85 13 " 20 15 " 22 38 " 30 44 " 42 89 " 79 111 " 102 119 to 105	ë.	131 to 125 118 '' 116 124 '' 143	119 " 124 123 " 136 121 " 118 121 " 118	122 " 98 122 " 98 107 " 110 130 " 88	117 '' 108 87 '' 100 127 '' 101	Pipe.	124 to 134 112 " 115 119 " 127 124 " 117
TUBERCULATED CAST-IRON PIPE	0.26 to 2.1 0.4 '' 3.7 0.47 '' 1.23 0.51 '' 1.19 0.38 '' 1.70 0.38 '' 1.70 1.0 '' 1.26 1.15 '' 1.97 4.08 1.0 to 5.0	l Riveted Pipe.	.30 to 2.79 .18 '' 2.33 .58 '' 6.14	4.70 " 10.01 1.30 " 10.52 4.60 " 10.10	0.56 " 5.70 3.71 " 3.91 0.96 " 4.99 2.04 " 6.06	1.57 " 3.80 1.12 " 4.80 1.00 " 4.50	WOODEN-STAVE	.70 to 1.53 3.46 "4.80 2.28 "4.70 1.20 "5.30
Ē.	\$04467 \$04467	-	222	J 70 12 4 4	× 01 42 25	888	-	111 111 48
-	1.41 3.13 4 4 6 6 6 9.57 16 24		1.05 3.00 7.25 7.25	10.93 11.22 12.67	14:70 36:00 42:00 47:4	72.24 72.24 102.96	-	14.05 44 54 72.36
-	Darcy. Brackett. """ "" "" Darcy. Forbes. Kuichling.	-	Darcy	H. Smith, Jr. Darcy. H. Smith, Jr.	Herschel. Kuichling. Herschel.	Marx, Wing and Hoskins Herschel		Adams Noble , Marx, Wing and Hoskins.

ŕ	<u>;</u>	•
:	Continue	
\ \(\(\)	VALUES—(•
	PIPE	

Remarks.		Long dimension horizontal	"		Cement-lined iron Experimental conduit		Boston main drainage sewer Milwaukee sewer		1-inch gas-pipe 1- ''		No fittings. Standard 1" pipe
Mean Value of c.		115	114		122 146		112 87		113 124		105 126 119 114 92
Range of c in H. & W. Formula.	Wooden Pipes.	122.0 to 112.0	116.8 " 106.8	PE.	127 to 118 148 '' 144	Tunnels.	113 to 111 95 '' 80	ON PIPE.	100 to 127 114" 134	GHT-IRON PIPE.	99.0 to 108.0 121.0 " 128.0 117.5 " 121.0 111.0 " 116.0 88.0 " 98.5
Range of Velocity, Feet per Second.	RECTANGULAR UNPLANED WOODEN PIPES.	1.67 to 6.37	1.23 '' 5.31	CEMENT PIPE.	1.49 to 4.04 2.78 " 6.60	CIRCULAR BRICK TUNNELS.	3.769 to 3.798 3.90 '' 7.00	NEW WROUGHT-IRON	1.03 to 1.58 0.96 "3.17	NEW GALVANIZED WROUGHT-IRON PIPE.	2.80 to 10.04 2.62 " 11.47 2.46 " 12.78 1.67 " 10.88 1.86 " 10.76
Num- ber of Obser- vations.	RECTA	∞	œ	_	111		10		2100	NEW	7 1 1 2 8 6
Diameter in Inches.		2.625×1.64 r = .505	$1.5/5 \times .984$ r = .303		20 31.50		90	-	0.628 1.054	-	1.042 0.850 0.626 0.486 0.350
Experimenter.		Darcy and Bazin	:	-	Fanning.		Clarke	-	H. Smith, Jr.	_	Saph and Schoder.

-	Seamless drawn ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,,	_		-					"A," extremely smooth "C," rubber-lined "D," "E," "I," "M," "M," "K," "mill hose "L," unlined linen hose
	131 145 138 138 138 137 137		137 134 130 123	_	130 133 122	-	95	•	143 140 132 135 106 89
Pipe.	129 to 146 142 " 149 130 " 146 131 " 145 130 " 144 130 " 147 140 " 150	IPE.	136 to 138 133 '' 135 122 '' 139 116 '' 129	ਬੰ	128 to 133 131 ** 135 119 ** 125	CORED.	95 to 95	ä	144 to 141 140 139 to 136 136 '' 130 114 '' 119 112 '' 101 93 '' 87
NEW BRASS PIPE.	0.95 to 14.97 0.65 '' 6.76 0.72 '' 5.47 0.36 '' 3.09 0.43 '' 3.64 0.38 '' 4.36 0.63 '' 4.95	NEW LEAD PIPE.	1.14 to 2.14 0.81 '' 1.46 0.82 '' 3.35 0.39 '' 7.56	GLASS PIPE.	1.40 to 2.65 1.95 ' 2.94 0.50 ' 6.92	NEW WOOD, BORED.	1.65 to 2.47	Fіне-ноя	23.55 23.55 23.55 23.50
_	39 10 10 10 10 10 10	- .	2010	-	010110	•	63	•	+の+++の%I-
-	9 5 7 5 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	- •	0.498 0.55 1.06 1.61	•	1.95	-	1.36	•	24282288 deletated al
-	Saph and Schoder.		Reynolds	•	H. Smith, Jr.	-	H. Smith, Jr		Premain

In a general way it may be said that for cast-iron pipe, very straight and smooth, c may be as high as 140, but for ordinary conditions 130 is a fair value for new pipe. As pipes rust and become dirty, the value of c decreases, as has been mentioned above. For new riveted steel pipe c is about 110.

In making estimates for pipe-lines where the carrying capacity after a series of years, rather than the value of the new pipe, is the controlling factor, a considerably lower value of c must be used, depending upon the amount of deterioration which is contemplated. A fair value for general computation is c=100 for cast-iron pipe and c=95 for steel pipe, but for small iron pipes a somewhat lower value of c should be taken. In the pipe tables the column of slopes for c=100 is printed in heavier-faced type than the rest, as these values are the ones which will probably be most often required. Lead, brass, tin, and glass, and other pipe presenting perfectly smooth surfaces, and perfectly straight, will give values of c up to 140. A very little falling off in the smoothness will reduce the value of c to 130 and 120, or For smooth wooden pipe or wooden-stave pipe 120 seems a fair value. For masonry conduits of concrete or plastered, with very smooth surfaces, when clean, values of c=140 may be observed. Generally such surfaces become slime-covered, reducing the value of c to 130 or less in a moderate length of time; and if the surfaces are only a little less smooth, say in such shape as is represented by ordinary good work, the value of c is reduced to 120. A conservative value for general use with first-class masonry structures is about 120. For brick sewers much lower values may be used, and c=100 seems safe. For vitrified pipe c=110 may be used. It must be understood that these values depend entirely upon the smoothness and regularity of the surfaces, and are likely to vary in individual cases.

This formula was designed primarily for computing the flow of water in pipes. It seems reasonably well adapted for computing the flow in open channels, and the slide-rules have been made so as to allow this application. A table has been prepared to show the application of this formula to the most reliable experiments upon open channels. From the data therein presented the investigator may determine for himself the probable accuracy to be obtained and the value of c which should be used in a given case.

TABLE NO. 2.—OPEN-CHANNEL VALUES.

Remarks.		Surface of pure cement '' '' brick laid flat '' '' gravel \$" to \$"' diam. '' '' '' 1\$" ''	"" "" unplaned plank "" "" "" "" "" "" "" "" "" "" "" "" ""	Surface covered with laths #" deep×1.1" wide, nailed #" apart around sides and bottom transversely to current	Surface covered with lath as	Surface of unplaned plank
Range of c in H. & W. Formula.		135 to 140 104 '' 110 76 '' 78 52 '' 66	109 (* 118 112 (* 118 113 (* 118 103 (* 118 110 (* 118 117 (* 119	89 " 101 83 " 92 82 " 87	60 " 67 54 " 61 54 " 58	112 ** 114 106 ** 117 106 ** 111
Range of v, Feet per Second.		.696 3 34 to 8 07 135 to 140 .7792.75 % 6.72104 % 110 .910 2.95 % 5.57 76 % 78 .987 1.79 % 4.90 52 % 66	.620 2. 08 ". 5.21 109 "630 3. 52 " 8. 57 113 "998 1. 80 " 4. 66 103 "686 2. 99 ". 7. 71 110 "621 3. 54 " 8. 74 117 ".	1.0761.65 ** 4.19 .790 2.50 ** 6.48 .726 2.85 ** 7.26	1.299 1.28 " 3.11 .9651.91 " 4.91 .885 2.21 " 5.57	.839 3.37 " 7.59 112 "700 2.85 " 6.48 106 "431 3.57 " 5.49 106 "
Range of r in Feet.	RECTANGULAR CONDUITS.	168 to .696 192 '' .779 357 '' .910 291 '' .987	240 '' .922 188 '' .727 147 '' .630 276 '' .998 172 '' .686 146 '' .621	302 '' 1.076 205 '' 790 182 '' 726	378 " 1.299 264 " .965 232 " .885	235 " .839 214 " .700 237 " .431
Slope, Feet per 1000 Feet.	CTANGU	4444 0.00 0.00	80.24 8.24 8.39 8.39	1.5 5.9 8.86	1.5 5.9 8.86	6.4
Mean Depth at Deepest Part of Section, Feet.	.B.	.18 to .91 .20 " 1.04 .41 " 1.30 .32 " 1.46	26 " 1.28 20 " 94 15 " 78 30 " 1.44 18 " 87 15 " 77	.33 '' — * .22 '' 1.05 .19 '' .94	.43 " 2.18 .29 " 1.38 .25 " 1.22	.27 '' 1.46 .26 '' 1.50 .34 '' .95
Width at Surface, Feet.		5.94 6.27 6.01 6.11	6.52 6.53 6.53 6.53	6.43 6.43 6.40	6.43 6.44 6.40	3.93 2.625 1.575
Num- ber of Obser- vations.		1212	112 122 7 - 7 - 7	777		11 9
Experimenter.		Darcy and Bazin, S. II '', '', '', S. III '', '', S. IV '', '', S. IV		AIX &	XXX XXXI XXXI XXXI XXXI XXXI	S. XVIII S. XIX S. XIX

* Conditions of flow irregular.

OPEN-CHANNEL VALUES—(Continued).

Remarks,		334 to 1.097 2.39 to 4.87 120 to 117 Sides at 45° for 1.64', then vertical above; bottom 3.28' wide 257 '' 837 3.58 '' 7.93 113 '' 120 One side vertical, other at 45°;	(bottom 3.1' wide Both sides at 45°, vertex down		.366 to 1.034 3.02 to 6.11 145 to 152 Eadius 2.05', surface pure	Radius 2.05', surface cement mixed with 4 of fine sand	Radius 2.295', partly planed plank	Radius 2.00', surface of small gravel \(\frac{2}{3}'' \) to \(\frac{2}{3}'' \) diameter set in cement		Very smooth wood Surface covered with cloth, lower corners rounded
Range of c in H. & W. Formula.	LANED.	120 to 117	114 " 118		145 to 152	132 " 141	121 " 129	66 ,, 06		115 to 132 124 " 133 57 " 82 45 " 71
Range of v, Feet per Second.	ONDUITS, UNP	.097 2.39 to 4.87 120 to 117	.839 4.13 " 7.75 114 " 118	ģ	3.02 to 6.11	2.87 " 5.66	2.61 " 5.54	2.17 " 3.95	DUIT.	.90 to 2.16 115 to 132 1.87 '' 3.56 124 '' 133 72 '' 1.88 57 '' 82 69 '' 2.23 45 '' 71
Range of r in Feet.	AR PLANK C	.334 to 1.097		SEMICIRCULAR CONDUITS.	.366 to 1.034	379 " 1.038 2.87 " 5.66 132 " 141	.390 " 1.148 2.61 " 5.54 121 " 129	.454 " 1.012 2.17 " 3.95 90 "	SMALL RECTANGULAR CONDUIT.	029 to .093 .030 '' .074 .038 '' .102 .031 '' .095
Slope, Feet per 1000 Feet.	LIANGUL	1.5	4.9	MICIRCI	1.5	1.5	1.5	1.5	RECTA	4.7 15.2 8.1 15.2
Mean Depth at Deepest Part of Section,	Trapezoidal and Triangular Plank Conduits, Unplaned.	.40 to 1.77	.92 '' 2.37	S	.59 to 2.08	.61 " 2.09	.63 '' 2.29	variable	SMALL	.036 to .215 4 .7 .037 '' .13415 .2 .048 '' .269 8.1 .036 '' .22615.2
Width at Surface, Feet.	Trapezo	6.56	3	•	variable	ï	ï	ť		0.328 0.328 0.312 0.312
Num- ber of Obser- vations.		12		•	12	12	13	01		0001
mter.	,	s. xxII	S. XXIII	•	S. XXIV	S. XXV	S. XXVI	S. XXVII		S. XXVIII S. XXXX S. XXXX
Experimenter		Darcy and Bazin, S.	:		÷	:	ä	=		::::
4		y and	ä		ä	:	*	:		::::
		Darc,	:		ä	ï	ä	*		****

OPEN-CHANNEL VALUES—(Continued).

		OIL	CHOUNT HENNING TO	ATOTAL TI	·(mananana)		
Experimenter.	No. of Observations.	Mean Depth, Feet.	Slope, Feet per 1000 Ft.	Range of r in Feet.	Range of v , ir. Feet per Second.	Range of c in H. & W. Formula.	Remarks.
				AQUEDUCTS.			
Fteley and Stearns	6.0	1.518 to 4.552	1928 to .1922	1.078 to 2.333	518 to 4.552 .1928 to .1922 1.078 to 2.333 1.827 to 2.926 135 to 132 505 '' 4.541 1893 '' 1889 1.071 '' 2.330 1.844 '' 2.937 137 '' 134	35 to 132 37 '' 134	Sudbury. Hard brick, fairly
33	n 00 (2.065 " 4.574 .0493 "	0493 (* 1860	1.400 (2.338	1.432 '' 2.909 1.	11 (* 134	bottom, 0.189. Horseshoe
	~ ~	2.002 ** 4.390	1998 '' 2600	1.366 '' 2.294	2.161 (3.3861)	34 131	section, 8.3' wide at bottom. Rad. $= 4.5'$. Invert 0.7' deep
	- ,	1.799 '' 3.878	2102 4913	1.251 2.151	.49131.251 2.151 2.448	132 (137)	Same conduit carefully cleaned.
;	E ;	0.719 1.410		0.435 1.010	1 11 (4 3 4 1	118 '' 130	Slope of bottom 0.16 New Croton. New York
Feley		12.8	0.133	3.93		122	Same conduit at point of maximum discharge
	_	-	- M	I Brick Sewers.	-		
Horton		1.02 to 2.89	0.500	0.688 to 1.539	1.99 to 3.44	116 to 121	Charlestown sewer 10 months in
	616	2.91 " 3.29	0.500	1.546 " 1.650	2.97 " 3.16 1 2.66 " 3.04 1	105 " 106	Do. 26 months in use Do. 4 years in use
	- c	1 02 " 4 62		0.619 '' 2.309		123 " 141	East Boston sewer 10 months in
	• 44	2.15 " 3.20		1.280 " 1.771	2.55 " 3.18 2.38 " 3.30	123 " 127 117 " 127	Do. 26 months in use Do. 4 years in use
		-	H	MARSEILLES AND CRAPONNE	CRAPONNE.		
Darcy and Bazin S. I	-	2.5× 7.4*	3.72	1.504	10.26	123	Nearly rectangular; brick side and cement bottom
Daumgarven j Ditto.	-		8.	1.774	5.55	134	Rectangular; smooth cut stone
Ditto.			23	.708	11.23	44	Noorly rootsnonler hamnered
Ditto.	-	0.0 × 3.5*	26	610. 188	7.58	62	stone, rather rough
Ditto.		1.5× 3.6*	14:	.835	8.36	r.	
Ditto.		4.5×19.7*	.43	2.871	2.54	65	Mud, grass, and weeds; trape- zoidal
				* Surface width.			

OPEN-CHANNEL VALUES—(Continued).

Remarks.	Solani Canal, Left. Masonry in good condition Solani Canal, Right. As last Solani Canal Main Sides	cla	(ureguar Betra. Similar to last Jasli. Similar (15 mile. old side. Farth beds	very rough Kamehera. Similar to last		Hammer-dressed, nearly rect-	5.91'. Some adhering slime	dressed, covered with moss & mud Rottom width 6 50'	Same as last, but cleaned. Bottom width 5.87
Range of c in H. & W. Formula.	77 to 123 83 " 86	46 ,, 26	61.6 to 61.8 74.4 ** 69.2	72 66.5 to 66	-	65 to 72	20 ,, 02	34 " 48	53 '' 66
Range of v, Feet per Second.	1.24 to 4.08 2.7 " 4.1	0.87 " 4.0	3.2	4.0 2.7 to 2.9	. s	.324 to .662 12.29 to 21.09	.424 " .852 9.04 " 15.08	.856 " 1.694 4.19 " 8.99	.703 " 1.491 5.66 " 11.26
Range of r	2.6 to 7.9 5.0 " 8.0	2.25 " 9.3	7.5	8.6 4.1 to 4.8	MASONRY SLUICEWAYS.	.324 to .662	.424 " .852	.856 " 1.694	.703 " 1.491
Slope, Feet per 1000 Feet.		088 "227 2.25 ". 9.3	.140 " .160		Mason	101.0	37.0	14.6	14.2
Area in Square Feet.						2.1 to 5.1	2.9 '' 7.0	8.9 ** 27.5	6.6 " 21.6
Num- ber of Obser- vations.	5 4	∞	88	- e		4	4	35	3
Experimenter.	Cunningham (Ganges Ca-) nals, Roorkee Expts.) } Ditto.	Ditto.	Ditto. Ditto.	Ditto. Ditto.	,	Darcy and Bazin, S. XXXII	" " S. XXXIII	" " S. XXXIV	" " S.XXXV

	104to110 Smooth masonry. Nearly rectangular. Bottom width 3.9'	Trapezoidal rough stone. Little vegetation. Bottom width 4.2	Trapezoidal with earth bottom and masonry sides. Bottom width 7 1'	Masonry in bad order. Vertical sides and circular invert. Bottom width 6.6	Similar to last, but in better order. Bottom width 6.2	Similar. Bottom width 6.6'	Earth, some vegetation. Form nearly arc of circle	Earth, no vegetation. Trape- zoidal. Bottom width 6.5'	Similar to last. Bottom width 6.3'	Trapezoidal in earth with vegetation. Bottom width 3.7'	Trapezoidal in stony earth. Little vegetation. Bottom width	Similar to last. Bottom width 4.1'	Similar to last. Bottom width .	Similar to last with vegetation. Bottom width 4.3'
-		$34~^{\prime\prime}~40$ Traper $^{\prime\prime}$	45 '' 58 Trap	65 " 94 Mass sic to to	80° $\left\{ $ Similar $\left\{ $ Similar $\left\{ $ Gim		37 '' 55) Earl 42 '' 56 \ ne	61 " 71 $\left\{ \begin{array}{c} \text{Eart} \\ \text{zo} \end{array} \right\}$	47 " 60 Simila 6.3"	33 '' 47 Tral	45 " $ 52$ Traj	42 '' 50 Sim	46 " 56 Sim	43 " 47 Simi
-	104	34	45	65	8	2	37 42	61	47	<u></u>	45	42	46	43
_	.406 to .766 5.73 to 8.75	10.5 " 24.6 .936 to .964 1.05 " 1.64 1.08 " 1.71	10.5 " 23.1 .525 " .487 1.00 " 1.67 1.01 " 1.74	9.7 " 21.1 .35 " .30 1.07 " 1.71 1.12 " 2.18	8.2 " 18.6 305 " 347 98 " 1.60 1.32 " 2.47	1.47 " 2.78	11.8 " 26.8 " 464 " " 4931.09 " 1.71 $.82$ " 1.68 10.1 " 25.9 " 55.6 " $.515$ $.99$ " 1.71 $.96$ " 1.75	.89 " 1.47	.82 " 1.41	.91 '' 1.65	9.5 " 22.9 792" .858 96 " 1.56 1.23 " 2.00	9.3 " 22.2 957 " 993 .96 " 1.54 1.24 " 1.96	11.3 " 27.2 445 " 455 1.04 " 1.71 96 " 1.51	4 11.6 " 28.7 .420 " .470 1.06 " 1.76 .89 " 1.39
ni.	992	1.64	1.67	1.71	8.	. 50	1.7	1.78	1.85	1.74	.56	5 7	17.1	92
Canals.	6 to .	5 '' 1	0 '' 1	7 '' 1	8 (1	8 " 1	 6	6 '' 1	5 '']	4 '']	6 '' 1	6 '' 1	4 '' 1	6 '' 1
ට .	<u>.</u>	1.0		1.0	<u>6.</u>	×.	2 20	<u></u>	1.0	1.1			1.0	-1.0
	_	96	.48	.30	.347	.68	.514	.27	.33(.62	858	.99	.45	.47(
	8.1	.936 to	.525 ''	.35	.302 ,,	.648	.464 .555	250 ''	.310 ''	,, 829	.792 ''	,, 226	.445 ''	.420 ''
	2.0 to 4.9	10.5 '' 24.6	10.5 " 23.1	9.7 " 21.1	8.2 " 18.6	7.4 " 16.6	11.8 " 26.8 10.1 " 25.9	10.9 " 30.8 250 " 275 .96 " 1.78	11.3 " 32.0 310 " .330 1.05 " 1.85	13.0 " 29.1 .678 " .622 1.14 " 1.74	9.5 " 22.9	9.3 " 22.2	11.3 " 27.2	11.6 " 28.7
•	4	4	4	4	4	4	44	4	4	4	4	4	4	4
•	Darcy and Bazin, S. XXXIX	S. XL	S. XLII	S. XLIV	S. XLV	S. XLVI	S. XLVII S. XLVIII	S. XLIX	S. L	S. XXXVI	S. XXXVII	S. XXXVIII	S. XLI	S. XLIII
	3azin,	:	:	:	:	:	::	:	:	:	:	÷	ä	E
	and E	:	:	:	z	÷	::	z	:	:	:	:	:	:
	Darcy a	÷	:	2	ä	:	::	÷	z	:	:	ä	ä	3

No tables to show the application of these results, that is to say, tables corresponding to the pipe tables, have been made for open channels. The variations in the conditions of depth, width, slope and character of bottom and sides are so enormously great that solution of each particular problem by the use of the slide-rule is the only practical way of handling the subject.

The slide-rule will also be found more closely applicable to actual conditions in pipes than any tables, because it gives at once values for conditions falling between the values which it is practicable to show in the tables, and its use is therefore to be recommended in all cases where close computations are desirable.

SMALL BRASS PIPES.

c = 130.

MAY ALSO BE USED FOR STRAIGHT LEAD, TIN, AND DRAWN-COPPER PIPES.

Diameter in	Gallons Daily for v=1		Loss of H	ead in Feet	per 1000 feet	of length.	
Inches.	Ft. per Second.	v = 0.5'	v = 1.0'	v = 2.0	v=3.0'	v=4.0'	v = 5.0'
0.03	3.2	1170	2350	4700	7050	9400	11700
0.04	5.6	660	1310	2620	3940	5250	6600
0.05	8.8	420	840	1680	2520	3370	4350
0.06	12.7	290	580	1170	1750	2340	3520
0.07	17.3	215	430	860	1290	1930	2950
0.08	22.6	164	<i>330</i>	660	990	1650	2500
0.09	28.5	130	260	5.20	840	1440	2200
0.10	35.3	105	210	420	750	1270	1940
0.11	42.7	87	174	<i>350</i>	670	1140	1730
0.12	51	73	146	293	605	1030	1560
0.14	69	54	108	239	505	860	1310
0.16	90 .	41	82	202	430	740	1120
0.18	114	<i>32</i>	65	178	375	640	980
0.20	141	26	52	157	333	570	860
0.22	171	21	43	141	300	510	770
0.24	203	18	<i>36</i>	127	270	460	700
0.26	238	15	32	116	245	418	640
0.28	277	13	30	106	225	382	580
0.30	317	12	27	98	209	354	540
0.35	432	9	23	83	175	299	450
0.40	564	7	19	70	149	252	385
0.45	714	5	17	- 61	130	220	335
0.50	880	4.15	15	54	114	195	295
0.55	1,070	3.75	13.4	48	102	174	. 265
0.60	1,270	3.35	12.1	44	92	157	240
0.65	1,490	3.07	11.0	40	84	144	220
0.70	1,730	2.80	10.1	36	77	132	200
0.75	1,990	2.59	9.4	34	71	121	184
0.80	2,260	2.40	8.7	31	66	113	170
0.85	2,550	2.23	8.1	29	62	105	159
0.90	2,860	2.10	7.6	27	58	98	148
0.95	3,180	1.96	7.1	26	54	92	139
1.00	3,525	1.85	6.7	24	51	87	131
1.10	4,250	1.65	6.0	21	46	78	117
1.20	5,080	1.50	5.4	19	41	70	106

Note.—Figures in italics are below the critical velocity and are computed by the formula $v = 475sd^2\left(\frac{t+10}{60}\right)$. t (temperature) is taken as 50° F.

SMALL PIPE.
WROUGHT-IRON-PIPE SIZES.

		Discha Gall	arge in		Loss of	Head in I	eet per 10	000 feet of	length.
Nom- inal Size, Inches.	Actual Diam- eter, Inches.	Per Minute.	Per 24 Hours.	Velocity, Feet per Second.	Very Smooth and Straight. $c=140$	Smooth New Iron. c=120	Ordinary Iron. c = 100	Old Iron. c=80	Very Rough. c=60
1	0.270	0.2	288	1.12	33	44	62	94	158
		0.4	5 76	2.24	118	158	220	335	570
		0.6	864	3.36	250	335	470	710	1210
		0.8	1,152	4.48	430	570	800	1210	2050
		1.0	1,440	5.60	650	860	1210	1830	3100
ł	0.364	0.5	720	1.54	42	56	78	118	200
		1.0	1,440	3.08	150	200	280	430	730
		1.5	2,160	4.62	320	425	600	910	1540
		2.0	2,880	6.16	550	730	1030	1550	2600
		2.5	3,600	7.70	830	1100	1530	2320	4000
ŧ	0.494	1	1,440	1.67	34	46	64	97	165
•	0.202	2	2,880	3.35	125	167	233	350	600
	1	3	4,320	5.02	260	350	490	740	1260
	1	4	5,760	6.70	450	600	840	1260	2150
		5	7,200	8.37	680	900	1260	1900	3250
1	0.623	1	1,440	1.05	11	15	21	31	53
-		2	2,880	2.10	40	, 53	74	112	192
	1	3	4,320	3.16	85	113	158	240	410
		4	5,760	4.21	145	192	270	410	700
		5	7,200	5.26	220	290	410	620	1050
		6	8,640	6.31	310	410	570	870	1470
		7	10,080	7.37	410	540	760	1150	1950
		8	11,520	8.42	520	700	980	1480	2500
	1	9	12,960	9.47	650	860	1210	1830	3100
		10	14,400	10.52	790	1050	1470	2230	3800
ŧ	0.824	.2	2,880	1.20	10	14	19	29	49
		3	4,320	1.80	22	29	41	61	105
		4	5,760	2.41	37	50	70	105	180
	1	5	7,200	3.01	56	75	105	159	270
		6	8,640	3.61	79	105	147	224	380
		8	11,520	4.81	135	180	250	380	650
	1	10	14,400	6.02	205	271	380	580	980
	}	12	17,280	7.22	285	380	530	800	1370
	· ·	15	21,600	9.02	430	570	800	1220	2030
		20	28,800	12.03	730	970	1360	2060	3500
	<u> </u>	<u> </u>		l	L				<u> </u>

SMALL PIPE.
WROUGHT-IRON-PIPE SIZES.

			arge in llons.		Loss of	Head in l	Feet per 10	000 feet of	length.
Nom- inal Size, Inches.	Actual Diam- eter, Inches.	Per Minute.	Per 24 Hours.	Velocity, Feet per Second.	Very Smooth and Straight. $c = 140$	Smooth New Iron. c=120	Ordinary Iron.	Old Iron. c=80	Very Rough. c=60
1	1.048	3	4,320	1.12	6.8	9.0	12.6	19.0	32
		.4	5,760	1.49	11.5	15.2	21.4	32.3	55
		5	7,200	1.86	17.5	23.2	32.5	49.1	84
		6	8,640	2.23	24.5	32.5	45.5	. 69	117
		8	11,520	2.98	42.0	55	78	117	200
		10	14,400	3.72	63	84	117	177	300
		12	17,280	4.46	88	117	· 164	250	420
		14	20,160	5.20	117	155	220	330	560
		16	23,040	5.95	150	200	280	420	720
		18	25,920	6.69	185	250	350	520	890
		20	28,800	7.44	22 6	301	420	640	1090
		25	36,000	9.30	340	455	640	960	1640
		30	43,200	11.15	480	640	890	1350	23 00
		35	50,400	13.02	640	850	1190	1800	3080
	•	40	57,600	14.88	820	1090	1520	2300	3900
11	1.380	4	5,760	0.86	3.0	4.0	5.7	8.6	14.5
		5	7,200	1.07	4.5	6.0	8.4	12.7	21.8
		6	8,640	1.29	6.4	8.6	12.0	18.2	31
		7	10,080	1.50	8.5	11.4	15.9	24	41
		8	11,520	1.72	11.0	14.5	20.3	31	53
		10	14,400	2.14	16.5	21.8	30 . 5	46	79
		12	17,280	2.57	23 .0	30.8	43	65	110
		14	20,160	3.00	30.8	41	57	87	148
		16	23,040	3.43	39.2	52	73	111	189
		18	25,920	3.86	49	65	91	137	235
		20	28,800	4.29	60	79	111	168	286
		25	36,000	5.36	89	119	166	251	430
		3 0	43,200	6.43	126	169	235	358	610
		35	50,400	7.51	168	223	312	470	800
		40	57,600	8.58	214	285	400	610	1030
		50	72,000	10.72	325	432	600	920	1560
- 1		60	86,400	12.87	450	610	850	1290	2200
1		70	100,800	15.01	610	810	1130	1700	2900
		80	115,200	17.16	780	1030	1450	2200	3700
1		90	129,600	19.30	960	1280	1800	2700	4600
		<u> </u>					1	!	

1½-INCH WROUGHT-IRON PIPE.

(Actual Diameter, 1.611.)

Discharge	in Gallons.		Los	s of Head in	Feet per 100	O feet of len	gth.
Per Minute.	Per 24 Hours.	Velocity, Feet per Second.	Very Smooth and Straight. c=140	Smooth New Iron. $c = 120$	Ordinary Iron.	Old Iron.	Very Rough.
4	5,760	0.63	1.42	1.87	2.62	4.0	6.
5	7,200	0.79	2.13	2.83	3.98	6.0	10.
6	8,640	0.94	2.98	3.98	5.6	8.4	14.
7	10,080	1.10	3.97	5.3	7.4	11.2	19.
8	11,520	1.26	5.1	6.8	9.5	14.3	24.
9	12,960	1.42	6.3	8.4	11.8	17.9	30.
10	14,400	1.57	7.7	10.2	14.3	21.7	3 6.
12	17,280	1.89	10.8	14.3	20.1	30.4	52
14	20,160	2.20	14.3	19.1	26.8	40.5	69
16	23,040	2.52	18.3	24.4	34.1	52	88
18	25,920	2.83	22.8	30.2	42.4	64	109
20	28,800	3.15	27.8	37	52	78	134
22	31,680	3.46	33.0	44	62	93	159
24	34,560	3.78	38.8	52	73	108	185
26	37,440	4.09	45.1	60	84	127	217
2 8	40,320	4.41	52	69	97	146	248
30	43,200	4.72	59	78	110	166	282
35	50,400	5.51	78	103	147	220	374
40	57,600	6.30	100	133	188	281	480
45	64,800	7.08	124	166	232	350	600
50	72,000	7.87	152	202	284	428	730
55	79,200	8.66	181	240	340	510	870
60	86,400	9.44	212	281	396	600	1020
65	93,600	10.23	246	328	459	700	1180
70	100,800	11.02	282	376	530	800	1360
75	108,000	11.80	321	427	600	900	1540
80	115,200	12.59	361	480	680	1020	1730
85	122,400	13.38	405	540	750	1140	1940
90	129,600	14.17	450	600	840	1260	2140
95	136,800	14.95	498	660	930	1400	2390
100	144,000	15.74	550	730	1020	1540	2620
110	158,400	17.31	650	870	1220	1840	3120
120	172,800	18.89	770	1020	1430	2170	3690
130	187,200	20.46	890	1180	1660	2500	4260
140	201,600	22.04	1020	1360	1900	2880	4890

2-INCH PIPE OR HOSE.

(Actual diameter, 2.00 ins.)

	arge in llons.				Loss of I	Head in I	eet per l	1000 feet	of lengt	h.
Per Minute.	Per 24 Hours.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Very Smooth and Straight Brass, Tin, etc. $c=140$	Ordinary Straight Brass, Tin, etc. c=130	Smooth New Iron. c=120	Ordinary Iron. $c = 100$	Old Iron.	Very Rough.	Badly Tuber-culated $c=40$
6	8,640	0.61	0.01	1.0	1.2	1.4	2.0	2.9	5.0	10.
8	11,520	0.82	0.01	1.8	2.0	2.4	3.3	5.0	8.6	18.
10	14,400	1.02	0.02	2.7	3.1	3.6	5.0	7.6	12.9	27
12	17,280	1.23	0.02	3.8		5.0	7.0	10.7	18.1	38
14	20,160	1.43	0.03	5.0	1	6.7	9.4	14.2	24.1	51
16	23,040	1.63	0.04	6.4	7.4	8.6	12.0	18.2	30.9	66
18	25,920	1.84	0.05	8.0	9.2	10.7	14.9	22.7	38.6	82
20	28,800	2.04	0.06	9.8	l .	12.9	18.2	27.5	46.8	99
25	36,000	2.55	0.10	14.8		19.6	27.3	41.6	71	150
3 0	43,200	3.06	0.15	20.7	23.8	27.3	38.4	58	99	210
35	50,400	3.57	0.20	27.5	31.5	3 6.6	51	78	132	280
40	57,600	4.08	0.26	35.1	40.2	46.8	66	99	168	359
45	64,800	4.60	0.33	43.8	50	58	82	123	210	446
50	72,000	5.11	0.40	53	61	71	99	150	257	540
55	79,200	5.62	0.49	64	73	84	118	179	305	640
60	86,400	6.13	0.58	74	86	99	139	210	359	760
65	93,600	6.64	0.68	86	99	115	161	244	416	880
70	100,800	7.15	0.79	99	114	132	184	280	477	1010
7 5	108,000	7.66	0.91	113	129	149	209	318	540	1150
80	115,200	8.17	1.04	127	146	169	237	358	610	1280
90	129,600	9.19	1.31	158	182	210	294	447	760	1610
100	1	10.21	1.62	192	220	256	358	540	920	1960
110		11.23	1.96	230	262	306	429	650	1110	2330
120	172,800	12.25	2.33	271	310	360	500	760	1300	2760
130	187,200	13.28	2.73	312	360	418	580	880	1510	3190
140	201,600	14.30	3.17	360	413	479	670	1020	1730	3670
150	1	15.32	3.64	407	465	540	760	1140	1950	4180
160	230,400	16.34	4.14	460	530	610	860	1290	2210	4690
170	244,800	17.36	4.67	520	590	690	960	1460	2480	5300
180	259,200	18.38	5.23	570	650	760	1070	1620	2730	5800
190	273,600	19.40	5.84	630	720	840	1180	1780	3030	6400
200		20.42	6.46	690	800	920		1960	3330	7100
220	316,800	22.47	7.82	830	950	1110	1540	2340	3990	8400
240	345,600	24.51	9.31	980	1120	1300	1820	2760	4700	9900
260		26.55	10.90	1130	1290	1510	2110	3190	5400	11500

2½-INCH PIPE OR HOSE.

(Actual diameter, 2.50 ins.)

Disch Gal	arge in llons.				Loss of I	Head in H	eet per 1	000 feet	of lengtl	h
Per Minute.	Per 24 Hours.	Veloc- ity in Feet per Second	Velocity Head, Feet.	Very Smooth and Straight Brass, Tin, etc. c=140	Ordinary Straight Brass, Tin, etc. c = 130	Smooth New Iron. c=120	Ordinary Iron.	Old Iron.	Very Rough.	Badly Tuber- culated.
8	11,250	0.52	0.00	0.6	0.7	0.8	1.1	1.7	2.9	6.1
10	14,400	0.65	0.01	0.9	1.0	1.2	1.7	2.6	4.3	9.2
12	17,280	0.78	0.01	1.3	1.4	1.7	2.4	3.6	6.1	12.9
14	20,160	0.92	0.01	1.7	2.0	2.3	3.2	4.7	8.2	17.4
16	23,040	1.05	0.02	2.2	2.5	2.9	4.1	6.2	10.5	22.2
18	2 5,920	1.18	0.02	2.7	3.1	3.6	5.0	7.6	12.9	27.3
20	28,800	1.31	0.03	3.3	3.8	4.3	6.1	9.2	15.7	33.2
25	36,000	1.63	0.04	4.9	5.7	6.6	9.2	13.9	23.7	50
30	43,200	1.96	0.06	6.9	8.0	9.2	12.9	19.5	33.2	70
35	50,400	2.29	0.08	9.2	10.6	12.3	17.2	26 .0	44.1	94
40	57,600	2.61	0.11	11.8	13.5	15.7	22.0	33.2	57	120
50	72,000	3.27	0.17	17.8	20.6	23.8	33 .2	51	86	182
60	86,400	3.92	0.24	24.9	28.7	33.2	46.5	70	120	254
7 0	100,800	4.58	0.33	33.2	38.1	44.2	62	94	160	338
80	115,200	5.23	0.43	42.5	48.8	56	79	120	204	433
90	129,600	5.88	0.54	53	61	70	98	149	254	540
100	144,000	6.54	0.66	64	74	86	120	182	309	660
120	172,800	7.84	0.95	90	103	120	168	254	433	920
140	201,600	9.15	1.30	120	138	159	223	339	580	1220
160	230,400	10.46	1.70	156	178	207	290	440	750	1570
180	259,200	11.76	2.15	191	219	254	357	540	920	1940
200	288,000	13.07	2.66	232	267	309	431	660	1120	2370
22 0	316,800	14.38	3.22	277	318	369	520	780	1330	2820
240	345,600	15.69	3.82	330	376	438	610	920	1570	3340
260	374,400	16.99	4.48	378	432	500	700	1070	1810	3860
280	403,200	18.30	5.20	432	497	580	810	1220	2080	4400
300	432,000	19.61	5.98	493	570	660	920	1390	2370	5000
320	460,800	20.92	6.80	560	640	740	1030	1570	2670	5700
340	489,600	22.22	7.68	620	710	820	1160	1750	2980	6400
3 60	518,400	23.53	8.60	690	790	920	1280	1940	3310	7100
380	527,200	24.84	9.60	780	890	1020	1420	2160	3670	7800
400	576,000	26.14	10.62	840	960	1120	1560	2370	4020	8600
42 0	604,800	27.45	11.70	920	1050	1220	1710	2590	4400	9300
440	633,600	28.76	12.85	1000	1150	1330	1860	2810	4800	10200
460	662,400	30.07	14.00	1110	1260	1460	2050	3100	5300	11200

(Actual diameter, 3.00 ins.)

Disch Ga	ar ge in llons.			:	Loss of I	Head in H	eet per l	1000 feet	of length	ı.
Per Minute.	Per 24 Hours.	Velocity in Feet per Second.	Veloc- ity Head, Feet.	Very Smooth and Straight Brass, Tin, etc. c=140	Ordinary Straight Brass, Tin, etc. c=130	Smooth New Iron. c=120	Ordinary Iron.	Old Iron.	Very Rough.	Badly Tuber-culated $c=40$
10	14,400	0.45	0.00	0.37	0.43	0.50	0.7	1.0	1.8	3.8
15	21,600	0.68	0.01	0.79	0.91	1.06	1.5	2.2	3.8	8.1
20	28,800	0.91	0.01	1.35	1.55	1.80	2.5	3.8	6.5	13.8
25	36,000	1.13	0.02	2.04	2.34	2.71	3.8	5.8	9.8	20.8
30	43,200	1.36	0.03	2.87	3.29	1	5.4	8.1	13.8	29.2
35	50,400	1.59	0.04	3.81	4.38	5.1	7.1	10.7	18.3	38.9
40	57,600	1.82	0.05	4.89	5.6	6.5	9.1	13.8	23.5	49.7
50	72,000	2.27	0.08	7.4	8.5	9.8	13.8	20.8	35.5	75
60	86,400	2.72	0.12	10.3	11.8	13.7	19.2	29.1	49.6	105
7 0	100,800	3.18	0.16	13.8	15.8	18.3	25.7	3 8.8	66	140
80	115,200	3.63	0.20	17.6	20.2	23.4	32 .8	49 .6	84	179
90	129,600	4.09	0.26	21.9	25.1	29.1	40.8	62	105	223
100	144,000	4.54	0.32	26.7	30.6	35.2	49.6	75	128	271
12 0	172,800	5.45	0.46	37.2	42.8	49.7	70	106	179	380
140	201,600	6.35	0.63	49.6	57	66	92	139	238	510
160	230,400	7.26	0.82	64	73	84	118	179	306	650
180	259,200	8.17	1.04	79	91	106	148	223	380	810
200	288,000	9.08	1.28	96	110	128	178	271	461	980
22 0	316,800	9.99	1.55	114	132	153	213	323	550	1170
240	3 45,6 00	10.89	1.84	134	154	179	251	380	650	1370
2 60	374,400	11.80	2.16	156	179	208	291	440	750	1590
280	403,200	12.71	2.51	179	206	238	334	510	860	1830
300	432,000	13.62	2.88	204	233	271	380	580	980	2080
32 0	460,800	14.52	3.28	229	263	306	428	650	1110	2330
340	489,600	15.43	3.71	257	294	342	479	720	1230	2610
360	518,400	ı	4.15	286	328	380	530	800	1370	2910
3 80	527,200		4.62	317	361	420	590	890	1520	32 10
400	576,000	1	5.11	348	399	461	650	980	1670	3520
420	604,800	19.06	5.64	380	436	510	710	1070	1830	3870
440	633,600	19.97	6.20	414	475	550	770	1170	1980	4220
4 60	662,400	20.88	6.78	449	520	600	840		2160	4570
480	691,200	21.79	7.38	488	560	650	910	1370	2330	4980
500	720,000	22.70	8.00	530	600	700	980	1480	2520	5400
5 50	792,000	24.96	9.70	620	720	830	1170	1770	3010	6400
600	864,000	27.23	11.50	740	840	980	1370	2070	3520	7400

Disc] Ga	harge in llons.	Veloc-	Walaa	1	Loss of H	ead in F	eet per 1	000 feet	of length	•
Per Minute.	Per 24 Hours.	ity in Feet per Second.	Velocity Head, Feet.	60 c=140	⊙ c = 130	(4) c=120	$ \begin{array}{c} 13 \\ c = 100 \end{array} $	26 c=80	(45) c=60	75) c=40
20	28,800	0.51	0.00	0.33	0.38	0.44	0.62	0.9	1.6	3.4
25	36,000	0.64	0.00	0.50	0.58	0.44	0.02	1.4	2.4	5.1
30	43,200	0.77	0.01	0.70	0.81	0.94	1.32	2.0	3.4	7.2
35	50,400	0.89	0.01	0.94	1.07	1.24	1.74	2.6	4.5	9.6
40	57,600	t	0.02	1.20	1.38	1.59	2.23	(5.8	12.2
5 0	72,000	1.28	0.03	1.82	2.08	2.41	3.39	5.1	8.8	18.5
60	86,400	1.53	0.04	2.53	2.91	3.38	4.72	7.2	12.2	25.9
70	100,800	1.79	0.05	3.38	3.88	4.50	6.3	9.5	16.3	34.4
80	115,200	2.04	0.06	4.32	4.97	5.8	8.1	12.2	20.8	44
90	129,600	2.30	0.08	5.4	6.2	7.2	10.0	15.2	25.9	55
100	144,000	2.55	0.10	6.5	7.5	8.8	12.2	18.5	31.3	66
120	172,800	ı	0.15	9.2	10.5	12.2	17.1	25.8	44	93
140	201,600	3.57	0.20	12.2	14.0	16.2	22.8	34.4	59	124
160	230,400	4.08	0.26	15.7	17.9	20.8	29.1	44	75	159
180	259,200	4.60	0.33	19.4	22.2	25.9	36.1	55	93	198
200	288,000	1	0.41	23.7	27.0	31.2	44	66	113	240
220	316,800	ŀ	0.49	28.1	32.2	37.3	52	79	135	287
240	345,600	1	0.58	33.0	37.9	44	62	93	158	337
260	374,400	1	0.69	38.3	44	51	72	108	184	391
280	403,200	7.15	0.79	44.0	50	59	82	124	210	448
300	432,000	7.66	0.91	50	57	67	93	141	240	510
320	460,800	8.17	1.04	56	65	75	105	158	271	580
340	489,600	8.68	1.17	63	72	84	117	178	303	640
360	518,400	9.19	1.31	70	80	93	131	197	337	710
400	576,000	10.21	1.62	85	98	113	160	241	410	870
450	648,000	1	2.05	107	122	141	198	299	510	1080
500	720,000	1	2.53	129	148	172	240	362	620	1320
550	792,000	1	3.06	153	177	205	287	433	740	1570
600	864,000	15.32	3.65	181	207	240	337	510	870	1840
650	936,000	16.59	4.28	209	240	279	390	590	1010	2130
7 00	1,008,000	1	4.96	240	276	320	449	680	1160	2450
750	1,080,000	1	5.70	272	312	362	510	770	1310	2790
800	1,152,000		6.48	308	352	410	570	870	1480	3120
850	1,224,000		7.30	343	395	458	640	970	1650	3510
900	1,296,000	22.98	8.20	382	439	510	710	1080	1840	3900

Discharg	e in Gallons.	Veloc-	Valor	I	oss of H	ead in F	eet per 10	000 feet o	of length.	
Per Minute.	Per 24 Hours.	ity in Feet per Second.	Veloc- ity Head, Feet.	00 c=140	o c=130	(4) c=120	(14) c=100	28 c=80	50 c = 60	87) c=40
30	43,200	0.49	0.00	0.24	0.27	0.31	0.44	0.67	1.1	2.4
40	57,600	0.65	0.01	0.40	0.46	0.54	0.75	1.14	1.9	4.1
50	72,000	0.82	0.01	0.61	0.70	0.81	1.13	1.72	2.9	6.2
60	86,400	0.98	0.02	0.86		1.13		2.41	4.1	8.7
70	100,800	1.14	0.02	1.14	1.31	1.52	2.12	3.21	5.5	11.7
80	115,200	1.31	0.03	1.46	1.67	1.94	2.71	4.11	7.0	14.8
90	129,600	1.47	0.03	1.82	2.08	2.41	3.39	5.1	8.7	18.
100	144,000	1.63	0.04	2.21	2.53	2.94	4.11	6.2	10.7	22.
120	172,800	1.96	0.06	3.09	3.54	4.11	5.8	8.7	14.8	31.
140	201,600	2.29	0.08	4.11	4.71	5.5	7.6	11.6	19.8	41.9
160	230,400	2.61	0.11	5.3	6.0	7.0	9.8	14.8	25.2	54
180	259,200	2.94	0.13	6.6	7.5	8.7	12.2	18.4	31.4	67
200	288,000	3.27	0.17	8.0	9.1	10.6	14.8	22.4	38.1	81
220	316,800	3.59	0.20	9.5	10.8	12.6	17.7	26.8	45.6	96
24 0	345,600	3.92	0.24	11.2	12.8	14.8	20.8	31.4	54	113
2 60	374,400	4.25	0.28	12.9	14.8	17.2	24.1	36.7	62	132
280	403,200	4.58	0.33	14.8	17.0	19.7	27.7	41.9	72	152
300	432,000	4.90	0.37	16.8	19.4	22.5	31.4	47.7	81	172
320	460,800	5.23	0.42	19.0	21.8	25.2	35.4	54	91	193
350	504,000	5.72	0.51	22.4	25.8	29.9	41.9	63	108	229
400	576,000	6.54	0.66	28.8	32.9	38.1	54	81	138	292
450	648,000	7.35	0.84	35.8	41.0	47.5	67	101	172	364
500	720,000	8.17	1.04	43.5	49.9	58	81	122	209	442
550	792,000	8.99	1.26	5 2	60	69	96	146	249	530
600	864,000	9.80	1.49	61	70	81	113	172	292	620
650	936,000	10.62	1.75	71	81	94	132	199	339	720
700	1,008,000	11.44	2.03	81	93	108	151	229	388	820
750	1,080,000	12.26	2.34	92	106	123	172	260	442	940
800	1,152,000	13.07	2.66	104	119	138	194	292	499	1060
850	1,224,000	13.89	2.99	117	133	154	217	328	560	1180
900	1,296,000	14.71	3.36	129	148	172	240	362	620	1320
950	1,368,000	15.52	3.74	143	163	190	267	402	690	1450
1000	1,440,000	16.34	4.15	157	180	209	292	443	750	160 0
1100	1,584,000	1	5.00	187	214	249	349	530	900	1910
1200	1,728,000	19.61	5.96	220	251	292	409	620	1480	2240

Dischar	ge in	Volce		I	oss of H	ead in F	eet per 1	000 feet	of length	•
Gallons per 24 Hours.	Cubic Feet per Second.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	@ c=140	⊙ c−130	(4) c-120	(15) c-100	30 c=80	(55) c = 60	95) c-4
50,000	0.0774	0.39	0.00	0.13	0.15	0.17	0.24	0.36	0.61	1
60,000	0.0928	0.47	0.00	0.18	0.20	0.24	0.33	0.51	0.86	1
70,000	0.1083	0.55	0.00	0.24	0.27	0.32	0.44	0.67	1.15	2
80,000	0.1238	0.63	0.01	0.30	0.35		0.57	0.86	1.46	3
	0.1392	0.71	0.01	0.38	0.43	I	0.71	1.07	1.83	3
100,000	0.1547	0.79	0.01	0.46	0.53	0.61	0.86	1.30	2.22	4
110,000	0.1702	0.87	0.01	0.55	0.63	0.73	1.03	1.55	2.65	5
120,000	l.	0.95	0.01	0.65	0.74	1				6
140,000		1.10	0.02	0.87	1	1		i .	-	8
160,000	0.2476	1.26	0.02	1.10	1.26	1.46	2.06	3.10	5.3	11
180,000	0.2785	1.42	0.03	1.37	1.57			3.88	6.6	14
200,000	0.3094	1.58	0.04	1.67			3.10	4.70	8.0	17
220,000	0.3404	1.73	0.05	1.99	2.29		1	5.6	9.6	20
240,000	0.3713	1.89	0.06			3.11	4.35	6.6	11.2	23
260,00 0	0.4023	2.05	0.07	2.71	3.10	3.60	5.0	7.6	13.0	27
280,000	0.4332	1	0.08	3.11	1		ł	8.8	15.0	31
300,000	1	1	0.09		1			10.0	17.0	36
350,000		2.76	0.12		1	6.3	8.8	13.3	22.5	48
400,000		3.15	0.15	1	6.9	8.0	11.3	17.0	29.0	62
450,000	0.696	3.55	0.19	7.5	8.6	10.0	14.0	21.2	36.0	76
500,000	0.774	3.94	0.24	9.1	10.4	12.1	16.9	25.6	43.8	92
550,000	0.851	4.33	0.29	1	12.4	14.4	20.1	30.5	52	110
600,000	0.928	4.73	0.35	1	14.6	17.0	23.8	36.0	61	130
650,000	1	5.12	0.41	1	16.9	19.6	27.5	41.6	71	150
700,000	1.083	5.52	0.47	17.0	19.5	22.6	31.6	48.0	82	173
800,000	1 -	6.30	0.62	1	24.9	28.9	40.4	61	104	221
900,000		7.09	0.78	1	30.9	35.8	50	76	129	274
,000,000	1 -	7.88	0.97		37.8	43.8	61	93	158	334
,100,000	1	8.67	1.17		45.1	52	78	111	189	400
,200,00 0	1.857	9.46	1.39	46.0	53	61	86	130	220	470
,400,000	1	11.03	1.89		70	82	114	173	295	620
,600,000	1	12.61	2.46	1	90	104	146	221	377	800
,800,000		14.18			112	130	182	275	470	990
,000,000	t .	15.76	3.85		137	159	222	337	570	1210
2,200,000	3.404	17.34	4.65	141	162	188	263	400	680	1440

Dischar	rge in	Veloc-	Walaa	_ 1	Loss of H	lead in F	eet per 1	000 feet	of length).
Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per Second.	Veloc- ity Head, Feet.	60 c = 140	⊚ c = 130	5 c-120	(10) c-110	(16) c=100	33 c=80	62 c-60
200,000	0.3094	0.89	0.01	0.41	0.47	0.55	0.64	0.77	1.16	1.98
220,000	0.3404	0.98	0.01	0.49	0.56	0.65	0.77	0.92	1.38	2.35
240,000	0.3713	1.06	0.02	0.58	0.66	0.77	0.90	1.07	1.62	2.78
260,000	0.4023	1.15	0.02	0.67	0.77	0.89	1.05	1 25	1.89	3.21
280,000	0.4332	1.24	0.02	0.77	0.88	1.02	1.20	1.43	2.16	3.69
3 00,000	0.4642	l .	0.03	0.87	1.00	1.16		-		
320,000	0.4951	1.42	0.03	0.98	1.13	1.31	1.54			
3 40,000	0.526	1.51	0.04	1.10	1.26	1.46				
36 0,000	0.557	1.60	0.04	1.22	1.40	1.62	1.91			5.9
3 80,000	0.588	1.68	0.04	1.35	1.55	1.80	2.11	2.51	3.80	6.5
400,000	0.619	1.77	0.05	1.48	1.70	1.97	2.32	2.76	4.20	7.1
450,000	0.696	1.99	0.06	1.85	2.11	2.45	2.89	3.43	5.2	8.9
500,000	0.774	2.22	0.08	2.25	2.58	2.99	3.50			10.7
5 50,000	0.851	2.44	0.09	2.68	3.07	3.55	4.19	l	7.6	12.9
600,000	0.928	2.66	0.11	3.14	3.61	4.19	4.91	5.9	8.9	15.1
650,000	1.006	2.88	0.13	3.64	4.18	4.84		6.8	10.3	17.5
700,000	1.083	3.10	0.15	4.19	4.80	5.6	6.5	7.8	11.8	20.0
750,000	1.160	3.32	0.17	4.73	5.4	6.3	7.4	8.8	13.3	22.8
800,000	1.238	3.55	0.20	5.3	6.1	7.1	8.4	9.9	15.1	25.7
900,000	1.392	3.99	0.25	6.7	7.6	8.9	10.4	12.4	18.8	32.0
1,000,000	1.547	4.43	0.30	8.1	9.3	10.8	12.7	15.1	23.0	39.0
1,100,000	1.702	4.88	0.37	9.6	11.1	12.8	15.1	18:0	27.2	46.2
1,200,000	1.857	5.37	0.44	11.3	13.0	15.1	17.7	21.1	32.0	54
1,300,000	2.011	5.76	0.52	13.1	15.1	17.5	20.5	24.5	37.0	63
1,400,000	2.166	6.20	0.60	15.1	17.3	20.0	23.5	28.1	42.5	72
1,500,000	2.321	6.65	0.69	17.0	19.5	22.6	26.7	31.8	48	82
1,600,000	2.476	7.09	0.78	19.2	22.0	25.5	3 0.0	35 .8	54	93
1,800,000	2.785	7.98	0.99	23 .8	27.2	31.6	37.1	44.2	67	114
2,000,000	3.094	8.86	1.22	29.0	33.3	38.7	45.4	54	82	140
2,200,000	3.404	9.75	1.47	34.9	40.0	46.2	54	65	98	167
2,400,000	3.713	10.64	1.76	41.0	47	55	64	77	116	198
2,600,000	4.023	11.52	2.06	47.5	55	63	74	89	134	229
2,800,000	4.332	12.41	2.39	1	62	73	· 85	102	153	261
3,000,000	4.642	13.30	2.74	62	71	83	97	116	175	300 `
3,200,000	4.951	14.18	3.12	70	80	93	109	130	197	336
		<u> </u>	<u> </u>	<u> </u>	1	ł		<u> </u>	<u> </u>	<u> </u>

Discha	rge in	Veloc-			Loss of H	lead in F	eet per 1	000 feet	of length	
Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per Second.	Velocity Head, Feet.	60 c=140	@ c=130	5 c=120	(10) c=110	(17) c=100	35 c=80	68 c=60
300,000	0.464	0.85	0.01	0.29	0.34	0.39	0.46	0.55	0.83	1.41
320,000	0.495	0.91	0.01	0.33	0.38	0.44	0.52	0.62	l .	1.59
340,000	0.526	0.96	0.01	0.37	0.42	0.49	0.58	0.69	1.04	
3 60,000	0.557	1.02	0.02	0.41	0.47	0.55	0.64	0.77	1.16	1.98
380,000	0.588	1.08	0.02	0.45	0.52	0.60	0.71	0.85		1
400,000	0.619	1.13	0.02	0.50	0.57	0.66	0.78	0.93	1.40	2.40
450,000	0.696	1.28	0.03	0.62	0.71	0.83	0.97	1.16	1.75	3.00
500,000	0.774	1.42	0.03	0.76	0.87	1.01	1.18	1.41	2.13	3.63
550,000	0.851	1.56	0.04	0.90	1.03	1.20	1.41	1.68	2.55	4.34
600,000	0.928	1.70	0.04	1.06	1.21	1.41	1.65	1.97	3.00	5.1
650,000	1.006	1.84	0.05	1.23	1.41	1.64	1.92	2.29	3.46	5.9
700,000	1.083	1.99	0.06	1.41	1.62	1.88	2.21	2.64	4.00	6.8
750,000	1.160	2.13	0.07	1.60	1.84	2.14	2.50	3.00	4.52	7.7
800,000	1.238	2.27	0.08	1.81	2.08	2.41	2.83	3.38	5.1	8.7
900,000	1.392	2.55	0.10	2.24	2.58	3.00	3.50	4.18	6.3	10.8
1,000,000	1.547	2.84	0.12	2.73	3.13	3.63	4.27	5.1	7.7	13.1
1,100,000	1.702	3.12	0.15	3.25	3.72	4.32	5.1	6.1	9.2	15.5
1,200,000	1.857	3.40	0.18	3.82	4.40	5.1	6.0	7.1	10.8	18.4
1,300,000	2.011	3.69	0.21	4.44	5.1	5.9	6.9	8.3	12.5	21.4
1,400,000	2.166	3.97	0.24	5.1	5.8	6.8	8.0	9.5	14.4	24.5
1,500,000	2.321	4.26	0.28	5.8	6.7	7.7	9.0	10.8	16.3	27.9
1,600,000	2.476	4.54	0.32	6.5	7.5	8.7	10.2	12.2	18.5	31.4
1,800,000	2.785	5.11	0.41	8.1	9.3	10.8	12.7	15.1	22.9	39.0
2,000,000	3.094	5.67	0.50	9.9	11.3	13.1	15.4	18.4	27.8	47.2
2,200,000	3.404	6.24	0.60	11.7	13.4	15.6	18.3	21.8	33.0	56
2,400,000	3.713	6.81	0.72	13.7	15.7	18.3	21.4	25.5	38.7	66
2,600,000	4.023	7.38	0.84	16.0	18.4	21.3	25.0	29 .9	45.0	77
2,800,000	4.332	7.94	0.98	18.3	21.0	24.3	28.6	34 .0	51	88 -
3,000,000	4.642	8.51	1.12	20.8	23.8	27.6	32.5	38 .6	59	100
3,200,000	4.951	9.08	1.28	23.5	27.0	31.2	36.8	43.8	66	113
3,400,000	5.26	9.65	1.44	l	30.2	35.0	41.2	49	74	127
3,600,000	5.57	10.21	1.62	I	33.5	38.9	45.5	54	82	140
3,800,000	5.88	10.78	1.80	32.5	37.2	43.1	51	60	92	156
4,000,000	6.19	11.35	2.00	35.5	40.8	47.3	56	66	100	171
4,500,000	6.96	12.77	2.52	44.3	51	59	69	83	125	213
		<u> </u>		<u>'</u>		1				

Discharg	ge in	Veloc-		I	oss of H	ead in F	eet per 1	000 feet	of length	ı.
Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Veloc- ity Head, Feet.	00 c=140	0c = 130	(B) c=120	(10) c=110	(17) c=100	26 c=90	37 c=80
100,000	0.155	0.20	0.00	0.02	0.02	0.02	0.02	0.03	0.04	0.04
200,000	0.309	0.39	0.00	0.06	0.07	0.08	0.09	0.11	0.13	0.16
300,000	0.464	0.59	0.01	0.12	0.14	0.16	0.19	0.22	0.27	0.34
400,000	0.619	0.79	0.01	0.20	0.24	0.27	0.32	0.38	0.47	0.58
50 0,000	0.774	0.99	0.02	0.31	0.36	0.41	0.48	0.58	0.71	0.88
600,000	0.928	1.18	0.02	0.44	0.50	0.58	0.68	0.81		
700,000	1.083	1.38	0.03	0.58	0.66	0.77	0.91	1.08	1.32	1.64
800,000	1.238	1.58	0.04	0.74	0.85	0.99	1.15	1.38	1.68	2.09
900,000	1.392	1.77	0.05	0.92	1.06	1.23	1.45	1.72	2.10	2.61
1,000,000	1.547	1.97	0.06	1.12	1.29	1.50	1.76	2.10	2.57	3.18
1,100,000	1.702	2.17	0.07	1.34	1.54	1.79	2.10	2.50	3.04	3.79
1,200,000	1	ì	0.09	1.58	1.81	2.10		2.94	1	1
1,300,000	1	2.56	0.10	1.83	2.10	2.43		1		i .
1 400,000		l	0.12	2.10	2.40	2.79		!	4.76	5.9
1,500,000		2.96	0.14	2.39	2.73	3.17		1	1	6.7
1,600,000	2.476	3.15	0.15	2.69	3.09	3.58	4.20	5.0	6.1	7.6
1,700,000	2.630	3.35	0.17	3.00	3.45	4.00	4.69	5.6	6.8	8.5
1,800,000	2.785	3.55	0.20	3.33	3.82	4.43	5.2	6.2	7.6	9.4
1,900,000	2.940	3.74	0.22	3.70	4.24	4.92	5.8	6.9	8.4	10.4
2,000,000	3.094	3.94	0.24	4.06	4.65	5.4	6.4	7.6	9.2	11.5
2,2 00,000	3.404	4.33	0.29	4.85	5.6	6.5	7.6	9.0	10.9	13.7
2,400,000	1	Į.	0.35	5.7	6.5	7.6	8.9	10.5	12.8	16.0
2 ,600,000	'	1	0.41	6.6	7.6	8.8	10.3	12.3	15.0	18.6
2,800,000		1	0.47	7.6	8.7	10.1	11.9	14.1	17.2	21.5
3,000,000	4.642	5.91	0.54	8.6	9.9	11.5	13.5	16.0	19.4	24.3
3,500,000	5.41	6.89	0.74	11.4	13.2	15.3	17.9	21.3	26.0	32.3
4,000,000	6.19	7.88	0.96	14.5	16.6	19.3	22.6	27.0	33.2	41.0
4,500,000	6.96	8.87	1.22	18.0	20.6	24.0	28.2	33.6	41.2	51
5,000,000	7.74	9.85	1.50	22.0	25.1	29.2	34.3	41.0	5 0.0	62
5,5 00,000	8.51	10.84	1.82	26.5	30.3	35.1	41.4	49.4	60	75
6,000,000	9.28	11.82	2.17	31.1	35.7	41.4	48.8	58	70	88
7,000,000	10.83	13.79	2.96	41.2	47.2	55	65	77	94	116
8,000,000	12.38	15.76	3.86	53	61	71	83	99	121	150
9,000,000	13.92	17.73	4.89	66	75	87	103	122	148	185
10,000,000	15.47	19.70	6.03	81	93	107	126 .	150	183	228
		<u> </u>								

Discharg	re in	Veloc-		1	loss of H	lead in F	eet per 1	.000 feet	of length	ւ
Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Veloc- ity Head, Feet.	6-140	⊚ ε-130	(5) c=120	(1) c=110	(18) c=100	⊙ c−90	39 c=80
200,000	0.309	0.22	0.00	0.014	0.016	0.019	0.022	0.026	0.03	0.04
400,000	0.619	0.44	0.00	0.051	0.058	0.068	0.080	0.095	0.12	0.14
600,000	0.928	0.66	0.01	0.108	0.124	0.143	0.169	0.201	0.24	0.30
800,000	1.238		0.01	0.183	0.210	0.242	0.287	0.340	0.41	0.52
1,000,000	1.547	1.11	0.02	0.278	0.319	1	1	0.52	0.63	0.78
1,200,000	1.857	1.33	0.03	0.389	0.446	0.52	0.61	0.72	0.88	1.09
1,400,000	2.166	1.55	0.04	0.52	0.60	0.69	0.81	0.96	1.18	1.47
1,600,000	2.476	1.77	0.05	0.66	0.76	0.88	1.03	1.23	1.50	1.87
1,800,000	2.785	1.99	0.06	0.82	0.95	1.09	1.28	1.53	1.87	2.32
:2,000,000	3.094	2.22	0.08	1.00	1.15	1.33	1.57	1.87	2.28	2.82
2,200,000	3.404	2.44	0.09	1.19	1.37	1.59	1.87	2,22	2.71	3.35
2,400,000	3.713	2.66	0.11	1.41	1.62	1.87	2.19	2.62	3.19	3.98
2,600,000	4.023	2.88	0.13	1.63	1.87	2.17	2.55	3.03	3.69	4.60
2,800,000	4.332	3.10	0.15	1.87	2.15	2.49	2.92	3.49	4.24	5.3
3,000,000	4.642	3.32	0.17	2.12	2.43	2.83	3.32	3.98	4.81	6.0
3,200,000		1	0.19	2.39	2.75	3.19	3.75	4.46	5.4	6.8
3,400,000	5.26	3.77	0.22	2.69	3.08	3.57	4.19	4.99	6.1	7.6
3,600,000	I .	3.99	0.25	2.98	3.42	3.97	4.65	5.6	6.8	8.4
3,800,000	1	4.21	0.28		3.78	4.38	5.1	6.2	7.4	9.3
4,000,000	6.19	4.43	0.31	3.61	4.15	4.80	5.6	6.8	8.2	10.2
4,500,000	1	4.99	0.39	1	5.2	6.0	7.0	8.4	10.2	12.7
5,000,000	i i	5.54	0.48	5.5	6.3	7.3	8.6	10.2	12.4	15.4
5,500,000		6.09	0.58	1	7.5	8.7	10.2	12.2	14.8	18.4
6,00 0,000		6.65	0.69	7.7	8.8	10.2	12.0	14.3	17.4	21.7
6,500,0 00	10.06	7.20	0.81	8.9	10.2	11.8	13.9	16.6	20.2	25.1
7,000,000	1	7.76	0.93		11.7	13.6	15.9	19.0	23.2	28.8
7,500,000	1	8.31	1.08	1	13.3	15.4	18.1	21.7	26.2	32.8
8,000,000	1	8.86	1.22		14.9	17.4	20.3	24.2	29.6	36.9
9,000,000	1	9.97	1.54	1	18.6	21.7	25.2	30.2	36.9	45.9
10,000,000	15.47	11.08	1.90	19.8	22.6	26.2	30.9	36.8	45.0	56
11,000,000	17.02	12.19	2.30	23.6	27.0	31.2	36.9	44.0	54	66
12,000,000	18.57	13.30	2.74	27.8	31.8	36.9	43.2	52	6 3	78
13,000,000	20.11	14.40	3.22	32.1	36.8	42.8	50	60	73	90
14,000,000	1	15.51	3.73	1	42.2	49.0	58	68	83	103
15,000,000	23.21	16.62	4.29	41.9	48.0	56	66	78	95	117

Dischar	ge in	Veloc-		I	oss of H	ead in F	eet per 1	000 feet	of length	•
Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Velocity Head, Feet.	00 c=140	(a) c=130	5 c=120	(1) c=110	(19) c=100	28) c=90	(41) c=80
400,000 600,000 800,000 1,000,000 1,200,000	0.928 1.238 1.547	0.43	0.00 0.00 0.00 0.01 0.01	0.017 0.037 0.062 0.094 0.131		$0.049 \\ 0.082 \\ 0.124$	0.057 0.097 0.146	0.115 0.174	0.039 0.083 0.140 0.211 0.297	0.103
1,400,000 1,600,000 1,800,000 2,000,000 2,500,000	2.476 2.785 3.094	1.13 1.28 1.42	0.02 0.02 0.03 0.03 0.05	0.174 0.223 0.278 0.339 0.51	$0.257 \\ 0.319$	$0.298 \\ 0.370$	$0.349 \\ 0.435$	0.416	0.396 0.51 0.63 0.76 1.16	0.491 0.63 0.78 0.96 1.44
3,000,000 3,500,000 4,000,000 4,500,000 5,000,000	5.41 6.19 6.96	2.13 2.48 2.84 3.19 3.55	0.07 0.10 0.13 0.16 0.20	0.72 0.95 1.22 1.52 1.84	0.82 1.09 1.39 1.74 2.11	0.95 1.27 1.62 2.02 2.45	1.12 1.49 1.90 2.38 2.88	1.33 1.78 2.28 2.83 3.43	1.61 2.16 2.77 3.44 4.18	2.02 2.69 3.44 4.29 5.2
5,500,000 6,000,000 6,500,000 7,000,000 7,500,000	9.28 10.06 10.83	3.90 4.26 4.61 4.96 5.32	0.24 0.28 0.33 0.38 0.44	2.20 2.59 3.00 3.43 3.90	2.52 2.97 3.43 3.95 4.48	2.92 3.44 3.99 4.58 5.2	3.43 4.03 4.68 5.4 6.1	4.09 4.81 5.6 6.4 7.3	4.98 5.8 6.8 7.8 8.8	6.2 7.3 8.4 9.7
8,000,000 8,500,000 9,000,000 9,500,000 10,000,000	13.15 13.92 14.70	5.67 6.03 6.38 6.74 7.09	0.50 0.56 0.63 0.71 0.78	4.39 4.91 5.5 6.0 6.6	5.1 5.6 6.3 6.9 7.6	5.8 6.6 7.3 8.0 8.9	6.9 7.7 8.6 9.4 10.4	8.2 9.2 10.2 11.3 12.4	10.0 11.2 12.4 13.7 15.1	12.4 13.8 15.4 17.1 18.7
11,000,000 12,000,000 13,000,000 14,000,000 15,000,000	18.57 20.11 21.66	7.80 8.51 9.22 9.93 10.64	0.94 1.12 1.32 1.53 1.76	7.9 9.4 10.8 12.4 14.1	9.1 10.7 12.4 14.2 16.2	10.6 12.4 14.4 16.5 18.8	12.4 14.6 16.9 19.4 22.0	14.8 17.4 20.1 23.1 26.2	18.0 21.1 24.4 28.1 32.0	22.4 26.2 30.4 35.0 39.8
16,000,000 17,000,000 18,000,000 19,000,000 20,000,000	26.30 27.85 29.40	11.35 12.06 12.77 13.47 14.18	2.00 2.25 2.53 2.82 3.13	15.8 17.7 19.7 21.8 24.0	18.2 20.4 22.7 25.0 27.6	21.1 23.8 26.2 29.1 32.0	24.8 27.9 30.9 34.1 37.5	29.6 33.1 36.8 40.7 44.8	36.0 40.2 44.7 49.5 54	44.8 50 56 62 68

Dischar	ge in	Veloc-]	Loss of H	lead in F	eet per 1	.000 feet	of length	ı.
Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Veloc- ity Head, Feet.	60 c-140		5 c-120	(1) c=110	(19) c-100	c=90	(42) c=80
500,000	0.774	0.25	0.00	0.011	0.012	0.014	0.017	0.020	0.024	0.030
1,000,000	1.547	0.49	0.00	0.038	0.044	0.051	0.060	0.072	0.087	0.108
1,500,000	2.321	0.74	0.01	0.082	0.093	0.108	0.128	0.152	0.185	0.230
2,000,000	ł	0.98	0.01	0.138	0.159	0.185	0.218	0.259	0.314	0.391
2,500,000	3.868	1.23	0.02	0.210	0.240	0.279	0.328	0.390	0.474	0.59
3,000,000	4.642	1.48	0.03	0.293	0.338	0.391	0.459	0.55	0.66	0.83
3,500,000	1	1.72	0.03	0.391	0.449	0.52	0.61	0.73	0.89	1.11
4,000,00 0	6.19	1.97	0.05	0.50	0.58	0.67	0.78	0.93	1.13	1.42
4,500,000		2.22	0.06	0.62	0.72	0.83	0.98	1.16	1.42	1.76
5,000,000	7.74	2.46	0.09	0.76	0.87	1.02	1.18	1.41	1.72	2.14
5,500,000	8.51	2.71	0.11	0.90	1.03	1.21	1.42	1.68	2.05	2.56
6,000,000	9.28	2.96	0.14	1.06	1.22	1.42	1.66	1.97	2.41	2.99
6,500,000	10.06	3.20	0.16	1.23	1.41	1.64	1.93	2.29	2.79	3.48
7,000,000	10.83	3.45	0.18	1.41	1.62	1.88	2.21	2.63	3.20	3.98
7,500,000	11.60	3.69	0.21	1.61	1.84	2.13	2.51	2.98	3.63	4.52
8,000,000	12.38	3.94	0.24	1.81	2.07	2.41	2.83	3.38	4.09	5.1
8,500,000	13.15	4.19	0.27	2.02	2.32	2.68	3.16	3.77	4.58	5.7
9,000,000	13.92	4.43	0.31	2.26	2.58	2.99	3.52	4.20	5.1	6.4
9,500,000	l .	4.68	0.34	2.48	2.85	3.31	3.89	4.62	5.6	7.0
10,000,000	15.47	4.92	0.38	2.73	3.12	3.63	4.28	5.1	6.2	7.7
11,000,000	17.02	5.42	0.46	3.26	3.74	4.33	5.1	6.1	7.4	9.2
12,000,000		5.91	0.54	3.82	4.39	5.1	6.0	7.1	8.7	10.8
13,000,000	l .	6.40	0.64	4.45	5.1	5.9	6.9	8.3	10.1	12.6
14,000,000		6.89	0.74	5.1	5.8	6.8	8.0	9.5	11.6	14.3
15,000,000	23.21	7.39	0.85	5.8	6.6	7.7	9.1	10.8	13.2	16.3
16,000,000	24.76	7.88	0.96	6.6	7.5	8.7	10.2	12.2	14.8	18.4
17.000,000	ı	8.37	1.09	7.3	8.4	9.7	11.4	13.6	16.6	20.7
18,000,000		8.86	1.22	8.1	9.3	10.8	12.7	15.2	18.4	22.9
19,000,000	(9.36	1.36	9.0	10.3	11.9	14.0	16.7	20.3	25.3
20,000,000	30.94	9.85	1.51	9.9	11.3	13.2	15.4	18.3	22.4	27.8
22,000,000	34.04	10.83	1.82	11.8	13.5	15.7	18.4	21.9	26.7	33.1
24,000,000	h'	11.82	2.17	13.8	15.8	18.4	21.7	25.9	31.2	3 9.0
26,000,000	40.23	12.80	2.55	16.1	18.4	21.3	25.0	29.9	36.4	45.2
28,000,000	43.32	13.79	2.96	18.3	21.1	24.5	28.8	34 . 2	41.9	52
30,000,000	46.42	14.77	3.38	20.9	24.0	27.9	32.8	39.0	47.5	59
	<u> </u>		<u> </u>	<u> </u>	<u> </u>	<u> </u>		l		l

Dischar	ge in	Volos		1	Loss of H	lead in F	eet per 1	.000 feet	of length	ı .
Gallons per 24 Hours.	Cubic Feet per Second.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	@0 c = 140	© c = 130	6 c=120	(12) c=110	(19) c = 100	30 c=90	(43) c=80
1,000,000	1.547	0.32	0.00	0.013	0.015	0.017	0.020	0.024	0.029	0.037
1,500,000	2.321	0.47	0.00	0.028	0.032	0.037	0.044	0.052	0.062	0.078
2,000,000	3.094	0.63	0.01	0.047	0.054	0.062	0.073	0.087	0.106	0.132
2,500,000	3.868	0.89	0.01	0.071	0.081	0.094	0.111	0.132	0.160	0.199
3,000,000	4.642	0.95	0.01	0.099	0.113	0.132	0.155	0.184	0.225	0.280
3,500,000	5.41	1.10	0.02	0.132	0.151	0.176	0.206	0.247	0.298	0.372
4,000,000	6.19	1.26	0.02	0.168	0.194	0.225	0.264	0.315	0.382	0.477
4,500,000	6.96	1.42	0.03	0.210	0.241	0.279	0.329	0.391	0.476	0.59
5,000,000	7.74	1.58	0.04	0.256	0.292	0.340	0.399	0.476	0.58	0.72
5,500,000	8.51	1.73	0.05	0.304	0.349	0.405	0.476	0.57	0.69	0.88
6,000,000	9.28	1.89	0.06	0.357	0.410	0.475	0.56	0.67	0.81	1.01
6,500,000	10.06	2.05	0.07	0.414	0.475	0.55	0.65	0.78	0.94	1.17
7,000,000	10.83	2.21	0.08	0.474	0.55	0.64	0.74	0.89	1.08	1.34
7,500,000	11.60	2.36	0.09	0.54	0.62	0.72	0.84	1.01	1.22	1.53
8,000,000	12.38	2.52	0.10	0.61	0.70	0.81	0.95	1.13	1.38	1.72
8,500 000	13.15	2.68	0.11	0.68	0.78	0.91、	1.07	1.27	1.54	1.92
9,000,000	13.92	2.84	0.13	0.76	0.87	1.01	1.18	1.42	1.72	2.14
.0,000,000	15.47	3.15	0.15	0.92	1.06	1.23	1.44	1.72	2.09	2.60
1,000,000	17.02	3.47	0.19	1.09	1.26	1.46	1.72	2.06	2.49	3.10
.2,000,000	18.57	3.78	0.22	1.28	1.47	1.72	2.02	2.41	2.92	3.64
3,000,000	20.11	4.10	0.26	1.50	1.72	1.98	2.34	2.79	3.40	4.21
4,000,000	21.66	4.41	0.30	1.72	1.97	2.28	2.69	3.20	3.89	4.85
5,000,000	23.21	4.73	0.35	1.95	2.24	2.60	3.06	3.64	4.43	5.5
6,000,000		5.04	0.40	2.20	2.52	2.93	3.45	4.10	4.99	6.2
7,000,000	26. 3 0	5.36	0.45	2.46	2.82	3.28	3.85	4.59	5.6	7.0
8,000,000	ı	5.67	0.50	2.74	3.14	3.63	4.28	5.1	6.2	7.7
9,000,000		5.99	0.56	3.02	3.47	4.01	4.72	5.6	6.8	8.6
0,000,000	1	6.30	0.62	3.33	3.81	4.44	5.2	6.2	7.6	9.4
2, 000,000	ı	6.93	0.75	3.96	4.55	5.3	6.2	7.4	9.0	11. 2
4,000,000	37.13	7.56	0.89	4.65	5.4	6.2	7.3	8.7	10.6	13.2
6,000,000	ı	8.20	1.04	5.4	6.2	7.2	8.4	10.1	12.3	15.3
8,000,000	1	8.83	1.21	6.2	7.1	8.3	9.7	11.6	14.1	17.5
0,000,000	•	9.46	1.39	7.1	8.1	9.4	11.0	13.2	16.0	19.8
5,000,000		11.03	1.89	9.4	10.8	12.6	14.7	17.5	21.3	26.4
0,000,000	61.9	12.61	2.47	12.0	13.8	16.0	18.8	22.4	27.2	33.9

Dischar	ge in	Veloc-		1	Loss of H	ead in F	eet per 1	000 feet	of length	•
Million Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per Second.	Veloc- ity Head, Feet.	60 c = 140	o c = 130	6 c = 12.)	(12) c = 110	20) c=100	30 c=90	(44) c=80
2	3.094	0.44	0.00	0.019	0.022	0.026	0.030	0.036	0.044	0.05
2.5	3.868	0.55	0.00	0.029	0.033	0.039	0.046	0.054	0.066	0.08
3	4.642	0.66	0.01	0.041	0.047	0.054	0.064	0.076	0.092	0.11
3.5	5.41	0.77	0.01	0.054	0.062	0.072	0.085	0.102	0.123	0.15
4	6.19	0.88	0.01	0.070	0.080	0.092	0.108	0.129	0.157	0.19
5	7.74	1.09	0.02	0.105	0.121	0.140	0.164	0.196	0.238	0.29
6	9.28	1.31	0.03	0.147	0.168	0.196	0.230	0.274	0.333	0.41
7	10.83	1.53	0.04	0.196	0.224	0.260	0.306	0.365	0.444	0.5
8	12.38	1.75	0.05	0.250	0.288	0.332	0.391	0.467	0.57	0.71
9	13.92	1.97	0.06	0.311	0.358	0.415	0.488	0.58	0.71	0.88
10	15.47	2.19	0.07	0.379	0.434	0.50	0.59	0.71	0.86	1.07
11	17.02	2.41	0.09	0.451	0.52	0.60	0.70	0.84	1.02	1.28
12	18.57	2.63	0.11	0.53	0.61	0.71	0.83	0.99	1.21	1.50
13	20.11	2.85	0.13	0.62	0.71	0.82	0.96	1.15	1.39	1.74
14	21.66	3.06	0.15	0.71	0.81	0.94	1.11	1.32	1.60	1.98
15	23.21	3.28	0.17	0.80	0.92	1.07	1.26	1.49	1.82	2.2
16	24.76	3.50	0.19	0.90	1.03	1.21	1.42	1.68	2.05	2.5
17	26.30	3.72	0.22	1.02	1.16	1.34	1.58	1.88	2.30	2.8
18	27.85	3.94	0.24	1.12	1.29	1.50	1.76	2.10	2.56	3.1
19	29.40	4.16	0.27	1.24	1.43	1.66	1.94	2.32	2.81	3.5
20	30.94	4.38	0.30	1.37	1.57	1.82	2.14	2.55	3.10	3.8
22	34.04	4.82	0.36	1.63	1.87	2.17	2.55	3.04	3.69	4.6
24	37.13	5.25	0.43	1.92	2.20	2.55	2.99	3.58	4.35	5.4
2 6	40.23	5.69	0.50	2.22	2.55	2.96	3.48	4.14	5.1	6.3
2 8	43.32	6.13	0.58	2.55	2.92	3.39	3.98	4.76	5.8	7.2
3 0	46.42	6.57	0.67	2.90	3.32	3.86	4.53	5.4	6.6	8.2
32	49.51	7.00	0.76	3.27	3.74	4.33	5.1	6.1	7.4	9.2
34	52.6	7.44	0.86	3.65	4.19	4.86	5.7	6.8	8.3	10.3
36	55.7	7.88	0.96	4.07	4.67	5.4	6.4	7.6	9.2	11.4
38	58.8	8.32	1.07	4.50	5.2	6.0	7.0	8.4	10.2	12.7
40	61.9	8.76	1.19	4.95	5.7	6.6	7.8	9.2	11.2	13.9
45	69.6	9.85	1.50	6.2	7.1	8.2	9.6	11.4	13.9	17.4
50	77.4	10.95	1.86	7.5	8.6	10.0	11.7	13.9	17.0	21.1
55	85.1	12.04	2.25	8.9	10.2	11.8	13.9	16.6	20.2	25.1
60	92.8	13.13	2.68	10.4	12.1	13.9	16.4	19.6	23.8	29.7

Discha	rge in	Veloc-		1	Loss of H	ead in F	eet per 1	000 feet	of length	•
Million Gallons per 24 Hours.	Cubic Feet per Second.	Feet per Second.	Veloc- ity Head, Feet.	60 c = 140	0 c=130	6 c=120	(12) c=110	20 c=100	30 c=90	(45) c=80
3	4.64	0.48	0.00	0.019	0.022	0.026	0.030	0.036	0.044	0.05
4	6.19	0.64	0.01	0.033	0.038	0.044	0.052	0.061	0.074	0.09
5	7.74	ı	0.01	0.050	0.057	0.066	0.078	0.092	0.113	0.14
6	9.28	0.96	0.01	0.070	0.080	0.092	0.108	0.129	0.158	0.19
7	10.83	1.13,	0.02	0.092		0.123	0.145	0.172	0.210	0.26
8	12.38	1.29	0.03	0.118	0.136	0.158	0.185	0.220	0.268	0.3
9	13.92	1.45	0.03	0.147	0.168	0.196	0.230	0.273	0.333	0.4
10	15.47	1.61	0.04	0.178	0.207	0.238	0.280	0.332	0.406	0.5
1 1	17.02	1.77	0.05	0.213	0.245	0.284	0.334	0.398	0.483	0.6
12	18.57	1.93	0.06	0.251	0.288	0.333	0.392	0.468	0.57	0.7
14	21.66	2.25	0.08	0.333	0.382	0.445	0.52	0.62	0.76	0.9
16	24.76	2.57	0.10	0.428	0.490	0.57	0.67	0.80	0.97	1.2
18	27.85	2.89	0.13	0.53	0.61	0.71	0.83	0.99	1.21	1.5
20	30.94	3.22	0.16	0.64	0.74	0.86	1.02	1.21	1.47	1.8
22	34.04	3.53	0.19	0.77	0.88	1.03	1.21	1.44	1.74	2.18
24	37.13	3.86	0.23	0.90	1.04	1.21	1.42	1.68	2.05	2.5
26	23ء40	4.18	0.27	1.05	1.21	1.39	1.64	1.96	2.38	2.9
28	43.32	4.50	0.31	1.21	1.38	1.61	1.88	2.25	2.74	3.4
30	46.42	4.82	0.36	1.37	1.57	1.83	2.14	2.56	3.10	3.8
32	49.51	5.15	0.41	1.54	1.77	2.06	2.41	2.88	3.50	4.3
34	52.6	5.47	0.46	1.73	1.98	2.29	2.70	3.21	3.91	4.8
3 6	55.7	5.79	0.52	1.92	2.20	2.56	3.00	3.58	4.35	5.4
3 8	58.8	6.11	0.58	2.12	2.43	2.82	3.31	3.95	4.80	6.0
40	61.9	6.45	0.64	2.33	2.68	3.10	3.64	4.35	5.3	6.6
42	65.0	6.75	0.71	2.56	2.92	3.40	3.99	4.76	5.8	7.2
44	68.1	7.08	0.78	2.78	3.19	3.70	4.36	5.2	6.3	7.8
46	71.2	7.40	0.85	3.02	3.48	4.02	4.71	5.6	6.8	8.5
48	74.3	7.72	0.93	3.28	3.76	4.36	5.1	6.1	7.4	9.2
50	77.4	8.04	1.01	3.52	4.05	4.70	5.5	6.6	8.0	10.0
55	85.1	8.84	1.21	4.21	4.82	5.6	6.6	7.8	9.6	11.8
60	92.8	9.65	1.45	4.94	5.7	6.6	7.7	9.2	11.2	13.9
65	100.6	10.45	1.70	5.7	6.6	7.6	9.0	10.7	13.0	16. 2
7 0	108.3	11.26	1.97	6.6	7.6	8.8	10.3	12.2	14.9	18.6
75	116.0	12.06	2.26	7.5	8.6	10.0	11.7	13.9	16.9	21.1
80	123.8	12.86	2.57	8.4	9.6	11.2	13.2	15.7	19.1	23.8

Dischar	rge in	Veloc-		:	Loss of F	lead in F	eet per 1	000 feet	of length	١.
Million Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Veloc- ity Head, Feet.	60 c=140	⊚ c = 130	6 c=120	(18) c=110	(a) c=100	30 c = 90	45 c=8
4	6.19	0.49	0.00	0.017	0.020	0.023	0.027	0.032	0.039	0.0
5	7.74	0.62	0.01	0.026	0.030	0.035	0.041	0.048	0.059	0.0
6	9.28	0.74	0.01	0.036	0.042	0.048	0.057	0.068	0.082	0.1
8	12.38	0.98	0.01	0.062	0.071	0.082	0.097	0.115	0.140	0.1
10	15.47	1.23	0.02	0.094	0.107	0.124	0.146	0.174	0.212	0.2
12	18.57	1.48	0.03	0.131	0.150	0.174	0.204	0.243	0.297	0.3
14	21.66	1.72	0.05	0.174	0.199	0.232	0.272	0.324	0.395	0.4
16	24.76	1.97	0.06	0.222	0.256	0.298	0.349	0.417	0.51	0.6
18	27.85	2.22	0.08	0.277	0.319	0.369	0.433	0.52	0.63	0.7
20	30.94	2.46	0.09	0.338	0.387	0.449	0.53	0.63	0.76	0.9
22	34.04	2.71	0.11	0.401	0.460	0.54	0.63	0.75	0.91	1.1
24	37.13	2.96	0.14	0.472	0.54	0.63	0.74	0.88	1.07	1.3
26	40.23	3.20	0.16	0.55	0.63	0.73	0.86	1.02	1.24	1.5
28	43.32	3.45	0.18	0.63	0.72	0.84	0.98	1.17	1.43	1.7
30	46.42	3.69	0.21	0.72	0.82	0.95	1.12	1.33	1.62	2.0
32	49.51	3.94	0.24	0.80	0.92	1.07	1.26	1.50	1.83	2.2
34	52.6	4.19	0.27	0.90	1.03	1.19	1.41	1.68	2.03	2.5
36	55.7	4.43	0.31	1.00	1.15	1.33	1.57	1.87	2.28	2.8
38	58.8	4.68	0.34	1,11	1.27	1.48	1.73	2.07	2.51	3.1
40	61.9	4.92	0.38	1.22	1.39	1.62	1.90	2.28	2.77	3.4
42	65.0	5.17	0.41	1.33	1.53	1.77	2.08	2.49	3.02	3.7
44	68.1	5.42	0.45	1.45	1.67	1.93	2.28	2.71	3.29	4.1
46	71.2	5.66	0.50	1.58	1.81	2.09	2.47	2.94	3.58	4.4
48	74.3	5.91	0.54	1.71	1.96	2.28	2.67	3.19	3.88	4.8
50	77.4	6.16	0.59	1.84	2.12	2.46	2.88	3.44	4.18	5.2
55	85.1	6.77	0.71	2.19	2.52	2.92	3.43	4.09	4.97	6.2
60	92.8	7.39	0.85	2.58	2.97	3.44	4.04	4.80	5.9	7.3
65	100.6	8.00	0.99	2.99	3.43	3.98	4.68	5.6	6.8	8.4
70	108.3	8.62	1.15	3.43	3.94	4.58	5.4	6.4	7.8	9.7
7 5	116.0	9.23	1.32	3.90	4.48	5.2	6.1	7.3	8.8	11.0
80	123.8	9.85	1.51	4.40	5.1	5.9	6.9	8.2	10.0	12.4
85	131.5	10.48	1.70	4.92	5.6	6.6	7.7	9.2	11.2	13.8
90	139.2	11.08	1.91	5.5	6.3	7.3	8.6	10.2	12.4	15.4
95	147.0	11.69	2.12	6.0	7.0	8.0	9.5	11.3	13.7	17.1
100	154.7	12.31	2.35	6.7	7.6	8.8	10.4	12.4	15.1	18.8

Dischar	ge in	Veloc-		, 1	Loss of H	lead in F	eet per 1	.000 feet	of lengtl	1.
Million Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Veloc- ity Head, Feet.	60 c = 140	c = 130	6 c-120	12 c=110	20 c=100	31 c = 90	46 c = 80
6	9.28	0.58	0.01	0.020	0.023	0.027	0.032	0.038	0.046	0.058
8	12.38	0.78	0.01	0.035	0.040	0.046	0.054	0.065	0.079	0.098
10	15.47	0.97	0.01	0.053	0.060	0.070	0.082	0.098	0.119	0.148
12	18.57	1.17	0.02	0.074	0.085	0.098	0.115	0.137	0.167	0.208
14	21.66	1.36	0.03	0.098	0.113	0.131	0.153	0.183	0.222	0.277
16	24.76	1.56	0.04	0.126	0.144	0.167	0.196	0.235	0.285	0.355
18	27.85	1.75	0.05	0.157	0.179	0.208	0.244	0.291	0.354	0.440
20	30.94	1.95	0.06	0.190	0.218	0.252	0.297	0.354	0.430	0.54
22	34.04	2.14	0.07	0.227	0.260	0.301	0.354	0.422	0.52	0.64
24	37.13	2.33	0.08	0.267	0.306	0.354	0.417	0.496	0.60	0.75
2 6	40.23	2.53	0.10	0.309	0.354	0.411	0.482	0.58	0.70	0.87
28	43.32	2.72	0.11	0.353	0.406	0.470	0.55	0.66	0.80	1.00
30	46.42	2.92	0.13	0.402	0.461	0.54	0.63	0.75	0.92	1.13
32	49.51	3.11	0.15	0.453	0.52	0.60	0.71	0.85	1.03	1.28
34	52.6	3.31	0.17	0.51	0.58	0.68	0.80	0.95	1.15	1.43
3 6	55.7	3.50	0.19	0.56	0.65	0.75	0.88	1.05	1.28	1.59
38	58.8	3.70	0.21	0.62	0.72	0.83	0.98	1.17	1.42	1.76
40	61.9	3.89	0.23	0.68	0.79	0.91	1.07	1.28	1.55	1.93
42	65.0	4.09	0.26	0.75	0.86	1.00	1.17	1.40	1.70	2.12
44	68.1	4.28	0.28	0.82	0.94	1.08	1.28	1.53	1.86	2.31
46	71.2	4.47	0.31	0.89	1.02	1.18	1.39	1.66	2.02	2.50
48	74.3	4.67	0.34	0.96	1.11	1.28	1.51	1.79	2.19	2.72
50	77.4	4.86	0.37	1.04	1.19	1.38	1.62	1.94	2.36	2.92
5 5	85.1	5.35	0.44	1.24	1.42	1.64	1.93	2.30	2.80	3.49
60	92.8	5.84	0.53	1.46	1.67	1.93	2.28	2.71	3.30	4.10
65	100.6	6.32	0.62	1.68	1.93	2.24	2.63	3.14	3.82	4.76
70	108.3	6.81	0.72	1.93	2.22	2.58	3.02	3.61	4.39	5.4
7 5	116.0	7.30	0.83	2.20	2.52	2.92	3.43	4.10	4.99	6.2
80	123.8	7.78	0.94	2.48	2.84	3.30	3.88	4.61	5.6	7.0
85	131.5	8.27	1.06	2.78	3.18	3.69	4.32	5.2	6.3	7.8
90	139.2	8.76	1.19	3.08	3.52	4.10	4.81	5.8	7.0	8.7
95	147.0	9.24	1.33	3.41	3.91	4.53	5.4	6.4	7.8	9.6
100	154.7	9.73	1.47	3.75	4.30	4.99	5.9	7.0	8.5	10.7
110	170.2	10.70	1.78	4.48	5.2	6.0	7.0	8.4	10.2	12.7
120	185.7	11.67	2.12	5.3	6.0	7.0	8.2	9.8	11.9	14.8

Dischar	ge in	Veloc-		:	Loss of H	Iead in F	eet per 1	.000 feet	of length	ı .
Million Gallons per 24 Hours.	Cubic Feet per Second.	ity in Feet per	Veloc- ity Head, Feet.	60). c=140	© c=130	6 c=120	(12) c=110	20 c=100	$\begin{array}{c} \boxed{31} \\ c = 90 \end{array}$	(47) c=80
4	6.19	0.32	0.00	0.006	0.007	0.008	0.009	0.011	0.013	0.016
6	9.28	0.47	0.00	0.012	0.014	0.016	0.019	0.023	0.028	0.03
8	12.38	0.63	0.01	0.021	0.024	0.028	0.033	0.039	0.047	0.059
10	15.47	0.79	0.01	0.032	0.036	0.042	0.049	0.059	0.072	0.089
12	18.57	0.95	0.01	0.044	0.051	0.059	0.069	0.082	0.100	0.124
14	21.66	1.10	0.02	0.059	0.068	0.078	0.092	0.109	0.133	0.166
16	24.76	1.26	0.02	0.075	0.086	0.100	0.117	0.140	0.171	0.212
18	27.85	1.42	0.03	0.094	0.107	0.124	0.146	0.174	0.212	0.263
20	30.94	1.58	0.04	0.113	0.131	0.152	0.178	0.212	0.258	0.320
22	34.04	1.73	0.05	0.136	0.156	0.181	0.212	0.253	0.308	0.381
24	37.13	1.89	0.06	0.159	0.183	0.212	0.249	0.298	0.361	0.449
2 6	40.23	2.05	0.07	0.185	0.212	0.247	0.289	0.346	0.419	0.52
28	43.32	2.21	0.08	0.212	0.243	0.282	0.331	0.395	0.480	0.60
30	46.42	2.36	0.09	0.241	0.277	0.320	0.377	0.449	0.55	0,68
32	'49.51	2.52	0.10	0.271	0.310	0.361	0.425	0.51	0.62	0.76
34	52.6	2.68	0.11	0.303	0.349	0.404	0.474	0.57	0.69	0.86
3 6	55.7	2.84	0.12	0.338	0.388	0.449	0.53	0.63	0.76	0.95
38	58.8	2.99	0.14	0.372	0.428	0.496	0.58	0.70	0.85	1.05
40	61.9	3.15	0.15	0.410	0.470	0.55	0.64	0.76	0.93	1.16
45	69.6	3.55	0.19	0.51	0.59	0.68	0.80	0.95	1.16	1.44
5 0	77.4	3.94	0.24	0.62	0.71	0.83	0.97	1.16	1.41	1.75
55	85.1	4.33	0.29	0.74	0.85	0.98	1.16	1.38	1.68	2.09
60	92.8	4.73	0.35	0.87	1.00	1.16	1.36	1.62	1.98	2.46
65	100.6	5.12	0.41	1.02	1.16	1.34	1.58	1.88	2.29	2.85
7 0	108.3	5.52	0.47	1.16	1.33	1.54	1.81	2.17	2.62	3.28
7 5	116.0	5.91	0.54	1.32	1.51	1.75	2.06	2.46	2.98	3.70
80	123.8	6.30	0.62	1.48	1.70	1.97	2.31	2.78	3.37	4.19
85	131.5	6.70	0.70	1.66	1.90	2.21	2.59	3.09	3.75	4.68
90	139.2	7.09	0.78	1.84	2.12	2.47	2.89	3.44	4.19	5.2
95	147.0	7.49	0.87	2.03	2.34	2.71	3.19	3.80	4.61	5.8
100	154.7	7.88	0.97	2.24	2.57	2.98	3.51	4.19	5.1	6.4
110	170.2	8.67	1.17	2.68	3.07	3.57	4.18	4.98	6.0	7.6
120	185.7	9.46	1.39	3.13	3.60	4.18	4.90	5.9	7.1	8.9
130	?01.1	10.24	1.63	3.63	4.18	4.84	5.7	6.8	8.3	10. 3
140	216.6	11.03	1.89	4.18	4.79	5.6	6.6	7.8	9.5	11.8

Dischar	ge in			1	Loss of H	ead in F	eet per 1	000 feet	of length	
Million Gallons per 24 Hours.	Cubic Feet per Second.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Extremely Smooth and Straight $c=140$	Very Smooth c=130	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New. c=110	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough c=80
8	12.38	0.52	0.00	0.013	0.015	0.017	0.021	0.024	0.030	0.037
10	15.47	0.65	0.01	0.020	0.023	0.026	0.031	0.037	0.045	0.056
12	18.57	0.78	0.01	0.028	0.032	0.037	0.043	0.052	0.063	0.078
14	21.66	0.91	0.01	0.037	0.042	0.049	0.058	0.069	0.084	0.104
16	24.76	1.04	0.02	0.047	0.054	0.063	0.074	0.088	0.107	0.133
18	27.85	1.17	0.02	0.059	0.068	0.078	0.092	0.109	0.133	0.166
20	30.94	1.30	0.03	0.071	0.082	0.095			1	0.202
22	34.04	1.43	0.03	0.085	0.098	0.113		0.158	0.193	0.240
24	37.13	1.56	0.04	0.100	0.115	0.133	0.157	0.187	0.228	0.283
26	40.23	1.69	0.04	0.116	0.133	0.154	0.182	0.217	0.262	0.328
28	43.32	1.82	0.05	0.133	0.153	0.178	0.208	0.248	0.302	0.376
30	46.42	1.95	0.06	0.152	0.173	0.201	0.237	0.282	0.343	0.427
32	49.51	2.08	0.07	0.171	0.196	0.227	0.267	0.318	0.388	0.480
34	52.6	2.21	0.08	0.191	0.219	0.254	0.298	0.356	0.432	0.54
36	55.7	2.34	0.09	0.212	0.243	0.282	0.331	0.396	0.481	0.60
3 8	58.8	2.47	0.10	0.235	0.269	0.312	0.368	0.438	0.53	0.66
40	61.9	2:60	0.11	0.258	0.296	0.344	0.403	0.481	0.59	0.73
45	69.6	2.93	0.13	0.320	0.368	0.427	0.50	0.60	0.73	0.90
50	77.4	3.26	0.16	0.390	0.448	0.52	0.61	0.73	0.88	1.10
55	85.1	3.58	0.20	0.466	0.53	0.62	0.73	0.87	1.06	1.32
60	92.8	3.91	0.24	0.55	0.63	0.73	0.86	1.02	1.24	1.54
65	100.6	4.23	0.28	0.64	0.73	0.84	0.99	1.18	1.44	1.79
70	108.3	4.56	0.32	0.73	0.84	0.97	1.14	1.36	1.65	2.06
75	116.0	4.88	0.37	0.83	0.95	1.10	1.29	1.54	1.87	2.33
80 ,	123.8	5.21	0.42	0.93	1.07	1.24	1.46	1.74	2.11	2.63
85	131.5	5.53	0.47	1.04	1.19	1.38	1.63	1.94	2.37	2.94
90	139.2	5.86	0.53	1.16	1.33	1.54	1.82	2.17	2.63	3.28
95	147.0	6.19	0.59	1.28	1.47	1.71	2.00	2.39	2.90	3.61
100	154.7	6.51	0.66	1.41	1.62	1.88	2.20	2.62	3.20	3.98
110	170.2	7.16	0.80	1.67	1.92	2.22	2.61	3.12	3.80	4.71
120	185.7	7.81	0.95	1.97	2.27	2.62	3.09	3.68	4.48	5.6
130	201.1	8.47	1.11	2.29	2.62	3.04	3.59	4.28	5.2	6.4
140	216.6	9.12	1.29	2.62	3.01	3.50	4.11	4.90	6.0	7.4
150	232.1	9.77	1.48	2.99	3.43	3.98	4.68	5.6	6.8	8.4
160	247.6	10.42	1.68	3.37	3.87	4.49	5.3	6.3	7.6	9.5

Dischar	ge in			:	Loss of E	lead in I	eet per 1	1000 feet	of length	1.
Million Gallons per 24 Hours.	Cubic Feet per Second.	Velocity in Feet per Second.	Velocity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth c=130	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New. c=110	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough
8	12.38	0.44	0.00	0.009	0.010	0.011	0.013	0.016	0.019	0.024
10	15.47	0.55	0.00	0.013	0.015	0.017	0.020		0.029	0.037
12	18.57	0.66	0.01	0.018	0.021	0.024	0.028	i .	0.041	0.051
14	21.66	0.77	0.01	0.024	0.028	0.032	0.038		0.055	0.068
16	24.76	0.88	0.01	0.031	0.035	0.041	0.048	0.058	0.070	0.088
18	27.85	0.98	0.02	0.038	0.044	0.051	0.060	0.072	0.087	0.108
20	30.94	1.09	0.02	0.047	0.054	0.062	0.073	0.087	0.106	0.132
22	34.04	1.20	0.02	0.056	0.064	0.074	0.087	0.104	0.126	0.157
24	37.13	1.31	0.03	0.066	0.075	0.087	0.103	0.122	0.148	0.188
26	40.23	1.42	0.03	0.076	0.087	0.102	0.118	0.142	0.172	0.215
28	43.32	1.53	0.04	0.087	0.100	0.116	0.136	0.162	0.197	0.246
· 3 0	46.42	1.64	0.04	0.099	0.113	0.132	0.155	0.185	0.225	0.279
32	49.51	1.75	0.05	0.112	0.128	0.148	0.174	0.208	0.252	0.315
34	52.6	1.86	0.05	0.125	0.143	0.166	0.195	0.232	0.282	0.351
- 36	55.7	1.97	0.06	0.138	0.159	0.185	0.217	0.259	0.315	0.391
3 8	58.8	2.08	0.07	0.153	0.176	0.204	0.240	0.287	0.348	0.432
40	61.9	2.19	0.07	0.169	0.193	0.225	0.263	9.315	0.382	0.476
45	69.6	2.46	0.09	0.210	0.241	0.280	0.329	0.391	0.477	0.59
5 0	77.4	2.74	0.12	0.255	0.292	0.340	0.399	0.477	0.58	0.72
5 5	81.5	3.01	0.14	0.304	0.349	0.405	0.476	0.57	0.69	0.86
60	92.8	3.28	0.17	0.358	0.410	0.476	0.56	0.67	0.81	1.02
65	100.6	3.56	0.20	0.414	0.475	0.55	0.65	0.78	0.94	1.17
7 0	108.3	3.83	0.23	0.476	0.55	0.64	0.74	0.88	1.08	1.34
7 5	116.0	4.10	0.26	0.54	0.62	0.72	0.84	1.01	1.23	1.53
80	123.8	4.38	0.30	0.61	0.70	0.81	0.96	1.14	1.38	1.72
90	139.2	4.92	0.38	0.76	0.87	1.01	1.18	1.42	1.72	2.14
100	154.7	5.47	0.47	0.92	1.07	1.23	1.44	1.72	2.10	2.60
110	170.2	6.02	0.56	1.10	1.27	1.47	1.72	2.05	2.49	3.10
120	185.7	6.57	0.67	1.28	1.48	1.72	2.01	2.40	2.92	3.64
130	201.1	7.11	0.79	1.50	1.72	1.99	2.34	2.79	3.40	4.21
140	216.6	7.66	0.91	1.72	1.97	2.29	2.69	3.20	3.90	4.84
150	232.1	8.21	1.05	1.95	2.24	2.60	3.05	3.62	4.41	5.5
160	247.6	8.76	1.19	2.20	2.52	2.92	3.43	4.10	4.99	6.2
170	263.0	9.30	1.34	2.46	2.82	3.28	3.85	4.59	5.6	7.0
180	278.5	9.85	1.51	2.73	3.13	3.63	4.29	5.1	6.2	7.8

Disch	arge in			:	Loss of I	Head in 1	Feet per 1	000 feet	of length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Velocity Head, Feet.	Ex- tremely Smooth and Straight	Very Smooth	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New.	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough.
10	6.46	0.30	0.00	0.004	0.004	0.005	0.006	0.007	0.009	0.011
15	9.69	0.45	0.00	0.008	0.009	0.011	0.013	0.015	0.019	0.023
20	12.93	0.60	0.01	0.014	0.016	0.019	0.022	0.026	0.032	0.040
25	16.16	0.75	0.01	0.021	0.024	0.028	0.033	0.040	0.048	0.060
3 0	19.39	0.90	0.01	0.030	0.034	0.040	0.047	0.056	0.068	0.084
35	22.62	1.05	0.02	0.040	0.046	0.053	0.062	0.074	0.090	0.112
40	25.85	1.21	0.02	0.051	0.058	0.068	0.080	0.095	0.116	0.144
45	29.08	1.36	0.03	0.064	0.073	0.084	0.099	0.118	0.144	0.178
5 0	32.32	1.51	0.04	0.077	0.088	0.102	0.120	0.143	0.174	0.218
5 5	35.55	1.66	0.04	0.092	0.106	0.122	0.144	0.172	0.208	0.259
60	38.78	1.81	0.05	0.108	0.124	0.144	0.169	0.201	0.245	0.304
65	42.01	1.96	0.06	0.126	0.144	0.167	0.196	0.233	0.284	0.354
7 0	45.24	2.11	0.07	0.143	0.164	0.190	0.223	0.268	0.325	0.404
7 5	48.47	2.26	0.08	0.163	0.186	0.217	0.253	0.303	0.369	0.459
80	51.7	2.41	0.09	0.184	0.211	0.246	0.288	0.343	0.419	0.52
85	54.9	2.56	0.10	0.205	0.236	0.272	0.321	0.382	0.467	0.58
90	58.2	2.71	0.11	0.228	0.262	0.304	0.358	0.426	0.52	0.64
95	61.4	2.86	0.13	0.252	0.290	0.337	0.396	0.471	0.57	0.72
100	64.6	3.01	0.14	0.278	0.319	0.369	0.432	0.52	0.63	0.78
110	71.1	3.32	0.17	0.331	0.379	0.440	0.52	0.62	0.75	0.94
120	77.5	3.62	0.20	0.389	0.446	0.52	0.61	0.72	0.88	1.09
130	84.0	3.92	0.24	0.450	0.52	0.60	0.71	0.84	1.02	1.27
140	90.5	4.22	0.28	0.52	0.59	0.69	0.81	0.96	1.17	1.46
150	96.9	4.52	0.32	0.59	0.68	0.78	0.92	1.09	1.33	1.66
160	103.4	4.82	0.36	0.66	0.76	0.88	1.03	1.23	1.50	1.87
170	109.9	5.12	0.41	0.74	0.85	0.99	1.16	1.38	1.68	2.09
180	116.3	5.43	0.46	0.82	0.94	1.09	1.28	1.54	1.87	2.32
190	122.8	5.73	0.51	0.91	1.04	1.22	1.43	1.70	2.07	2.58
200	129.3	6.03	0.56	1.00	1.15	1.33	1.57	1.87	2.27	2.82
220	142.2	6.63	0.68	1.19	1.37	1.59	1.87	2.22	2.70	3.38
240	155. 1	7.23	0.81	1.40	1.61	1.87	2.20	2.62	3.19	3.97
2 60	168.0	7.84	0.95	1.63	1.87	2.17	2.54	3.04	3.69	4.59
2 80	181.0	8.44	1.11	1.87	2.14	2.49	2.92	3.49	4.23	5.3
300	193.9	9.04	1.27	2.12	2.43	2.82	3.31	3.96	4.80	6.0
3 20	206.8	9.64	1.44	2.39	2.75	3.19	3.74	4.45	5.4	6.8

Disch	arge in				Loss of I	Head in I	Feet per 10	000 feet o	f length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Extremely Smooth and Straight $c = 140$	Very Smooth c=130	Good Massonry Aqueducts. $c = 120$	Riveted Steel Pipe, New.	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough.
10	6.46	0.26	0.00	0.003	0.003	0.004	0.004	0.005	 σ.00€	0.008
15	9.69	0.39	0.00	0.006	0.007	0.001	0.009	0.011	0.000	0.008
20	12.93	0.52	0.00	0.010	0.011	0.013	0.015	0.018	0.022	0.018
2 5	16.16	0.65	0.01	0.015	0.017	0.020	0.023	0.028	0.034	0.042
30	19.39	0.78	0.01	0.021	0.024	0.028	0.033	0.039	0.047	0.059
3 5	22.62	0.91	0.01	0.028	0.032	0.037	0.043	0.052	0.063	0.078
40	25.85	1.04	0.02	0.036	0.041	0.047	0.056	0.066	0.080	0.100
45	29.08	1.17	0.02	0.044	0.051	0.059	0.069	0.082	0.100	0.124
50	32.32	1.30	0.03	0.054	0.062	0.072	0.084	0.100	0.122	0.152
5 5	35.55	1.43	0.03	0.064	0.074	0.086	0.100	0.119	0.145	0.181
60	38.78	1.56	0.04	0.075	0.086	0.100	0.118	0.141	0.171	0.212
65	42.01	1.69	0.04	0.087	0.100	0.117	0.136	0.163	0.198	0.247
70	45.24	1.82	0.05	0.100	0.114	0.133	0.157	0.187	0.228	0.282
80	51.7	2.08	0.07	0.128	0.147	0.171	0.200	0.239	0.290	0.361
90	58.2	2.34	0.09	0.159	0.183	`0. 2 12	0.249	0.297	0.361	0.450
100	64.6	2.60	0.11	0.193	0.222	0.257	0.302	0.361	0.439	0.55
110	71.1	2.86	0.13	0.231	0.265	0.307	0.361	0.430	0.52	0.65
120	77.5	3.12	0.15	0.272	0.311	0.361	0.424	0.51	0.62	0.76
130 '	84.0	3.38	0.18	0.314	0.361	0.419	0.492	0.59	0.71	0.89
140	90.5	3.64	0.21	0.361	0.414	0.480	0.56	0.68	0.82	1.04
150	96.9	3.90	0.24	0.410	0.470	0.54	0.64	0.77	0.93	1.16
160	103.4	4.16	0.27	0.461	0.53	0.62	0.72	0.86	1.04	1.30
170	109.9	4.42	0.30	0.52	0.60	0.69	0.81	0.96	1.17	1.46
180	116.3	4.68	0.34	0.58	0.66	0.76	0.90	1.07	1.30	1.62
190	122.8	4.94	0.38	0.64	0.73	0.84	0.99	1.18	1.44	1.79
200	129.3	5.20	0.42	0.70	0.80	0.93	1.09	1.30	1.58	1.97
220	142.2	5.72	0.51	0.83	0.96	1.11	1.30	1.55	1.88	2.35
240	155.1	6.24	0.60	0.98	1.12	1.30	1.53	1.82	2.21	2.77
2 60	168.0	6.76	0.71	1.13	1.30	1.51	1.77	2.11	2.57	3.20
280	181.0	7.28	0.82	1.30	1.49	1.73	2.03	2.42	2.96	3.68
300	193.9	7.80	0.94	1.48	1.70	1.97	2.32	2.77	3.37	4.19
320	206.8	8.31	1.08	1.67	1.91	2.22	2.61	3.11	3.78	4.70
3 40	219.7	8.83	1.21	1.87	2.14	2.48	2.92	3.48	4.22	5.3
3 60	232.7	9.35	1.36	2.08	2.38	2.76	3.25	3.88	4.70	5.9
380	245.6	9.87	1.52	2.29	2.63	3.03	3.59	4.29	5.2	6.5

Disch	arge in				Loss of I	Head in I	Feet per 10	000 feet o	of length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth c=130	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New.	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough.
15	9.69	0.34	0.00	0.004	0.005	0.006	0.007	0.008	0.009	0.012
20	12.93	0.45	0.00	0.007	0.008	0.009	0.011	0.013	0.016	0.020
25	16.16	0.57	0.00	0.011	0.012	0.014	0.017	0.020	0.024	0.030
30	19.39	0.68	0.01	0.015	0.017	0.020	0.023	0.028	0.034	0.042
35	22.62	0.79	0.01	0.020	0.023	0. 02 6	0.031	0.037	0.045	0.056
40	25.85	0.91	0.01	0.026	0.029	0.034	0.040	0.048	0.058	0.072
45	29.08	1.02	0.02	0.032	0.036	0.Q42	0.050	0.059	0.072	0.090
50	32.32	1.13	0.02	0.038	0.044	0.051	0.060	0.072	0.087	0.108
60	38.78	1.36	0.03	0.054	0.062	0.072	0.084	0.101	0.122	0.152
. 70	45.24	1.58	0.04	0.072	0.083	0.096	0.113	0.134	0.163	0.202
80	51.7	1.81	0.05	0.092	0.105	0.122	0.143	0.171	0.208	0.259
90	58.2	2.04	0.06	0.114	0.131	0.152	0.179	0.213	0.260	0.322
100	64.6	2.26	0.08	0.139	0.160	0.186	0.218	0.260	0.316	0.392
110	71.1	2.49	0.10	0.166	0.190	0.221	0.259	0.309	0.376	0.468
120	77.5	2.72	0.11	0.194	0.222	0.259	0.303	0.361	0.440	0.55
130	84.0	2.94	0.13	0.226	0.259	0.301	0.353	0.421	0.51	0.64
140	90.5	3.17	0.16	0.259	0.298	0.344	0.404	0.481	0.59	0.73
150	96.9	3.40	0.18	0.294	0.338	0.391	0.460	0.55	0.67	0.83
160	103.4	3.62	0.20	0.332	0.381	0.442	0.52	0.62	0.76	0.94
170	109.9	3.85	0.23	0.371	0.425	0.493	0.58	0.69	0.84	1.04
180	116.3	4.07	0.26	0.413	0.472	0.55	0.64	0.77	0.94	1.17
190	122.8	4.30	0.29	0.457	0.52	0.61	0.72	0.85	1.03	1.29
200	129.3	4.53	0.32	0.50	0.58	0.67	0.78	0.94	1.14	1.42
220	142.2	4.98	0.39	0.60	0.69	0.80	0.94	1.12	1.36	1.69
240	155.1	5.43	0.46	0.70	0.81	0.94	1.10	1.31	1.59	1.98
000	100.0	F 90	0.54	0.00	0.94	1.08	1.27	1.52	1.84	2.30
260	168.0	5.89	0.54 0.62	0.82	1.07	1.08	1.46	1.74	2.11	2.62
280	181.0 193.9	$6.34 \\ 6.77$	$0.02 \\ 0.72$	1.07	1.21	1.41	1.65	1.97	2.40	2.98
300 320	206.8	7.25	0.72	1.19	1.37	1.58	1.86	2.22	2.70	3.38
340	219.7	7.70	0.92	1.33	1.53	1.78	2.09	2.49	3.02	3.78
		0.15	1 00	1 40	1 71	1.00	0.00	2.78	3.39	4.20
3 60	232.7	8.15	1.03	1.49	1.71	1.98	2.32	3.08	3.73	4.65
3 80	245.6	8.60	1.15	1.65	1.89 2.08	$\frac{2.20}{2.41}$	$2.58 \\ 2.82$	3.38	3.73 4.10	5.1
400	258.5	9.05			2.08	2.41	3.10	3.70	4.50	5.6
420	271.5 284.4	9.51 9.96	1.40 1.54	1.98 2.17	2.48	2.89	3.39	4.02	4.90	6.1
440	401.1	9.50	1.04	2.11	2.33	2.00	0.00	1.02	1.00	J.1

Disch	arge in			1	Loss of E	lead in F	eet per 10	000 feet o	f length.	
Cubic Feet per econd.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Velocity Head, Feet.	Extremely Smooth and Straight $c=140$	Very Smooth c=130	Good Ma- sonry Aque- ducts.	Riveted Steel Pipe, New.	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough.
15	9.69	0.30	0.00	0.003	0.003	0.004	0.005	0.006	0.007	0.009
20	12.93	0.40	0.00	0.005	0.006	0.007	0.008	0.010	0.012	0.015
30	19.39	0.60	0.01	0.011	0.013	0.015	0.017	0.020	0.025	0.031
40	25.85	0.80	0.01	0.019	0.021	0.025	0.029	0.035	0.042	0.053
50	32.32	0.99	0.02	0.028	0.032	0.037	0.043	0.052	0.063	0.078
60	38.78	1.19	0.02	0.039	0.045	0.052	0.061	0.073	0.089	0.110
70	45.24	1.39	0.03	0.052	0.060	0.070	0.082	0.097	0.118	0.147
80	51.7	1.59	0.04	0.067	.0.077	0.089	0.104	0.124	0.152	0.188
90	58.2	1.79	0.05	0.083	0.095	0.111	0.130	0.155	0.188	0.234
100	64.6	1.99	0.06	0.101	0.116	0.135	0.158	0.188	0.229	0.286
110	71.1	2.19	0.07	0.121	0.138	0.161	0.188	0.226	0.273	0.341
120	77.5	2.39	0.09	0.143	0.163	0.190	0.222	0.267	0.322	0.401
130	84.0	2.59	0.10	0.165	0.189	0.220	0.259	0.308	0.374	0.466
140	90.5	2.79	0.12	0.189	0.218	0.251	0.297	0.352	0.429	0.54
150	96.9	2.99	0.14	0.216	0.248	0.288	0.338	0.401	0.489	0.61
160	103.4	3.19	0.16	0.242	0.279	0.322	0.380	0.451	0.55	0.68
170	109.9	3.39	0.18	0.271	0.311	0.361	0.425	0.51	0.62	0.76
180	116.3	3.59	0.20	0.302	0.348	0.402	0.471	0.56	0.68	0.86
190	122.8	3.78	0.22	0.332	0.381	0.442	0.52	0.62	0.85	0.94
200	129.3	3.98	0.25	0.366	0.420	0.488	0.57	0.68	0.83	1.03
22 0	142.2	4.38	0.30	0.437	0.50	0.58	0.68	0.81	0.99	1.23
240	155.1	4.77	0.36	0.52	0.59	0.68	0.80	0.95	1.17	1.45
260	168.0	5.17	0.42	0.60	0.68	0.79	0.93	1.11	1.34	1.68
280	181.0	5.57	0.48	0.68	0.78	0.91	1.07	1.27	1.55	1.93
300	193.9	5.97	0.55	0.78	0.89	1.03	1.22	1.45	1.76	2.19
320	206.8	6.37	0.63	0.87	1.00	1.16	1.36	1.63	1.98	2.46
340	219.7	6.76	0.71	0.98	1.12	1.30	1.53	1.82	2.22	2.76
360	232.7	7.16	0.80	1.08	1.25	1.44	1.70	2.02	2.47	3.07
380	245.6	7.56	0.89	1.20	1.38	1.60	1.88	2.24	2.72	3.39
400	258.5	7.96	0.98	1.32	1.52	1.76	2.07	2.48	3.00	3.73
420	271.5	8.36	1.09	1.44	1.66	1.92	2.27	2.69	3.28	4.08
440	284.4	8.75	1.19	1.58	1.81	2.10	2.47	2.93	3.58	4.45
460	297.3	9.15	1.30	1.71	1.96	2.28	2.68	3.19	3.88	4.82
480	310.2	9.55	1.42	1.86	2.13	2.48	2.90	3.46	4.21	5.2
500	323.2	9.95	1.54	2.00	2.29	2.66	3.12	3.72	4.52	5.6

Disch	arge in				Loss of I	Head in 1	Feet per 1	000 feet o	of length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Velocity in Feet per Second.	Veloc- ity Head, Feet.	Extremely Smooth and Straight $c=140$	Very Smooth c=130	Good Massonry Aqueducts. $c=120$	Riveted Steel Pipe, New. $c=110$	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough.
20	12.93	Q.35	0.00	0.004	0.004	0.005	0.006	0.007	0.009	0.011
30	19.39	0.53	0.00	0.008	0.009	0.011	0.013	0.015	0.018	0.023
40	25.85	0.70	0.01	0.014	0.016	0.018	0.022	0.026	0.031	0.039
5 0	32.32	0.88	0.01	0.021	0.024	0.028	0.033	0.039	0.047	0.059
60	3 8.78	1.06	0.02	0.029	0.034	0.039	0.046	0.055	0.066	0.082
70	45.24	1.23	0.02	0.039	0.045	0.052	0.061	0.073	0.088	0.110
80	51.7	1.41	0.03	0.050	0.057	0.066	0.078	0.093	0.113	0.141
90	58.2	1.59	0.04	0.062	0.071	0.083	0.097	0.116	0.141	0.175
100	64.6	1.76	0.05	0.076	0.086	0.101	0.118	0.141	0.171	0.212
110	71.1	1.94	0.06	0.090	0.103	0.119	0.141	0.167	0.204	0.253
120	77.5	2.11	0.07	0.106	0.122	0.141	0.165	0.197	0.239	0.298
130	84.0	2.29	0.08	0.123	0.141	0.163	0.192	0.228	0.278	0.345
140	90.5	2.47	0.09	0.141	0.162	0.187	0.220	0.262	0.319	0.398
150	96.9	2.64	0.11	0.159	0.182	0.212	0.249	0.298	0.361	0.450
160	103.4	2.82	0.12	0.180	0.207	0.239	0.281	0.335	0.408	0.51
170	109.9	3.00	0.14	0.201	0.231	0.268	0.315	0.375	0.456	0.57
180	116.3	3.17	0.16	0.224	0.258	0.299	0.350	0.417	0.51	0.63
190	122.8	3.35	0.17	0.248	0.283	0.330	0.388	0.461	0.56	0.70
200	129.3	3.52	0.19	0.272	0.311	0.361	0.424	0.51	0.62	0.77
220	142.2	3.88	0.23	0.323	0.371	0.431	0.51	0.60	0.74	0.92
240	155.1	4.23	0.28	0.381	0.438	0.51	0.60	0.71	0.86	1.07
2 60	168.0	4.58	0.33	0.441	0.51	0.59	0.69	0.82	1.00	1.25
280	181.0	4.93	0.38	0.51	0.58	0.68	0.79	0.94	1.14	1.43
300	193.9	5.29	0.44	0.58	0.66	0.77	0.90	1.08	1.31	1.63
320	206.8	5.64	0.49	0.65	0.74	0.86	1.02	1.22	1.47	1.83
34 0	219.7	5.99	0.56	0.73	0.84	0.97	1.13	1.36	1.65	2.05
3 60	232.7	6.34	0.62	0.81	0.93	1.07	1.27	1.51	1.83	2.28
38 0	245.6	6.70	0.70	0.89	1.03	1.18	1.39	1.67	2.02	2.52
400	258.5	7.05	0.77	0.98	1.13	1.31	1.53	1.83	2.23	2.77
420	271.5	7.40	0.85	1.08	1.23	1.43	1.68	2.00	2.44	3.02
440	284.4	7.75	0.93	1.17	1.34	1.56	1.83	2.19	2.67	3.30
460	297.3	8.10	1.02	1.27	1.46	1.69	1.98	2.38	2.89	3.59
480	310.2	8.46	1.11	1.38	1.58	1.83	2.16	2.58	3.12	3.89
500	323.2	8.81	1.20	1.48	1.71	1.98	2.32	2.78	3.38	4.20
5 50	355.5	9.69	1.46	1.77	2.02	2.36	2.76	3.30	4.01	4.99
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Disch	arge in				Loss of I	Head in H	eet per 10	000 feet o	f length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth c=130	Good Massonry Aqueducts. $c = 120$	Riveted Steel Pipe, New. c=110	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Roug
20	12.93	0.31	0.00	0.003	0.004	0.004	0.005	0.006	0.008	0.00
30	19.39	0.31	0.00	0.006	0.007	0.004	0.003	0.000		
40	25.85	0.63	0.00	0.000	0.007	0.003	0.016	0.011	0.014	0.01
50	32.32	0.79	0.01	0.016	0.012	0.014	0.016	0.019	$0.024 \\ 0.036$	0.02
60	38.78	0.73	0.01	0.010	0.025	0.021	0.025	0.023	0.050	0.04
70	45.24	1.10	0.02	0.029	0.034	0.039	0.046	0.055	0.067	0.08
80	51.7	1.26	0.02	0.038	0.043	0.050	0.059	0.070	0.086	0.10
90	58.2	1.41	0.03	0.047	0.054	0.062	0.073	0.087	0.106	0.13
100	64.6	1.57	0.04	0.057	0.066	0.076	0.089	0.106	0.128	0.16
110	71.1	1.73	0.05	0.068	0.078	0.090	0.106	0.126	0.153	0.19
120	77.5	1.89	0.06	0.080	0.092	0.106	0.124	0.148	0.181	0.22
130	84.0	2.04	0.07	0.092	0.106	0.123	0.144	0.172	0.209	0.26
140	90.5	2.20	0.08	0.107	0.122	0.141	0.166	0.198	0.240	0.29
150	96.9	2.36	0.09	0.122	0.138	0.161	0.188	0.225	0.273	0.34
160	103.4	2.52	0.10	0.136	0.156	0.181	0.212	0.252	0.309	0.38
180	116.3	2.83	0.12	0.169	0.194	0.225	0.264	0.314	0.382	0.47
200	129.3	3.14	0.15	0.206	1	0.272	0.321	0.382	0.466	0.58
220	142.2	3.46	0.19	0.246	0.281	0.326	0.382	0.457	0.56	0.70
240	155.1	3.77	0.22	0.289	0.330	0.382	0.450	0.54	0.65	0.8
2 60	168.0	4.09	0.26	0.335	0.384	0.445	0.52	0.62	0.76	0.94
280	181.0	4.40	0.30	0.382	0.440	0.51	0.60	0.72	0.87	1.08
300	193.9	4.72	0.35	0.436	0.50	0.58	0.68	0.81	0.99	1.23
320	206.8	5.03	0.39	0.491	0.56	0.66	0.77	0.92	1.12	1.38
3 40	219.7	5.34	0.44	0.55	0.63	0.73	0.86	1.03	1.24	1.5
360	232.7	5.66	0.50	0.61	0.70	0.81	0.96	1.14	1.38	1.72
380	245.6	5.97	0.55	0.68	0.78	0.90	1.06	1.26	1.53	1.90
400	258.5	6.29	0.61	0.74	0.85	0.99	1.16	1.38	1.68	2.0
420	271.5	6.60	0.68	0.81	0.93	1.08	1.27	1.51	1.84	2.2
440	284.4	6.92	0.74	0.88	1.02	1.18	1.38	1.65	2.00	2.4
460	297.3	7.23	0.81	0.96	1.11	1.28	1.50	1.78	2.18	2.7
480	310.2	7.55	0.88	1.04	1.19	1.38	1.63	1.94	2.36	2.9
5 00	323.2	7.86	0.96	1.12	1.28	1.49	1.75	2.09	2.54	3.1
550	355.5	8.65	1.16	1.34	1.54	1.78	2.09	2.50	3.03	3.7
600	387.8	9.43	1.38	1.57	1.81	2.09	2.47	2.93	3.58	4.4
650	420.1	10.22	1.62	1.82	2.09	2.42	2.85	3.40	4.12	5.2

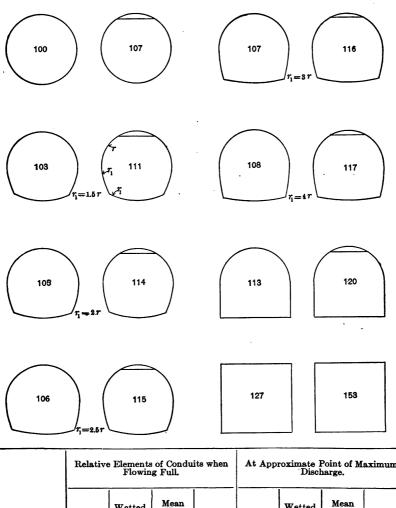
Disch	arge in] :	Loss of E	lead in F	eet per 10	000 feet o	f length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Velocity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth c=130	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New.	Steel Pipe 10 Years Old, Brick Sewers, c=100	Rough.	Very Rough.
30	19.39	0.42	0.00	0.004	0.005	0.006	0.007	0.009	0.011	0.013
40	25.85	0.56	0.00	0.008	0.009	0.011	0.013	0.015	0.018	0.023
50	32.32	0.71	0.01	0.012	0.014	0.016	0.019	0.023	0.028	0.034
60	38.78	0.85	0.01	0.017	0.019	0.023	0.027	0.032	0.038	0.048
7 0	45.24	0.99	0.02	0.023	0.026	0.030	0.035	0.042	0.051	0.064
80	51.7	1.13	0.02	0.029	0.033	0.038	0.045	0.054	0.066	0.082
90	58.2	1.27	0.03	0.036	0.041	0.048	0.056	0.067	0.082	0.102
100	64.6	1.41	0.03	0.044	0.050	0.059	0.068	0.082	0.099	0.123
110	71.1	1.55	0.04	0.052	0.060	0.069	0.082	0.097	0.118	0.147
120	77.5	1.69	0.04	0.061	0.070	0.082	0.096	0.114	0.138	0.173
130	84.0	1.83	0.05	0.071	0.081	0.094	0.112	0.132	0.161	0.200
140	90.5	1.98	0.06	0.081	0.094	0.108	0.127	0.152	0.185	0.230
150	96.9	2.12	0.07	0.093	0.106	0.123	0.145	0.173	0.210	0.261
160	103.4	2.26	0.08	0.104	0.120	0.139	0.163	0.195	0.237	0.294
180	116.3	2.54	0.10	0.130	0.149	0.173	0.202	0.242	0.295	0.367
200	129.3	2.82	0.12	0.158	0.181	0.210	0.248	0.294	0.358	0.446
220	142.2	3.10	0.15	0.188	0.217	0.251	0.294	0.351	0.428	0.53
240	155.1	3.38	0.18	0.221	0.253	0.294	0.347	0.412	0.50	0.62
260	168.0	3.67	0.21	0.257	0.294	0.341	0.401	0.479	0.58	0.72
280	181.0	3.95	0.24	0.294	0.338	0.391	0.460	0.55	0.67	0.83
300	193.9	4.23	0.28	´0.333	0.382	0.445	0.52	0.62	0.76	0.94
320	206.8	4.52	0.32	0.377	0.432	0.50	0.59	0.70	0.86	1.07
340	219.7	4.80	0.36	0.421	0.482	0.56	0.66	0.79	0.96	1.19
360	232.7	5.08	0.40	0.469	0.54	0.63	0.73	0.88	1.07	1.32
3 80	245.6	5.36	0.45	0.52	0.60	0.69	0.81	0.97	1.17	1.46
400	258.5	5.64	0.50	0.57	0.65	0.76	0.89	1.07	1.29	1.61
420	271.5	5.93	0.55	0.62	0.72	0.83	0.98	1.17	1.42	1.76
440	284.4	6.21	0.60	0.68	0.78	0.90	1.07	1.27	1.54	1.92
460	297.3	6.49	0.65	0.74	0.85	0.98	1.16	1.38	1.67	2.08
480	310.2	6.77	0.71	0.80	0.92	1.07	1.25	1.48	1.82	2.26
500	323.2	7.06	0.77	0.86	0.99	1.14	1.34	1.61	1.95	2.43
550	355.5	7.76	0.94	1.03	1.18	1.37	1.61	1.92	2.33	2.90
600	387.8	8.47	1.11	1.21	1.38	1.61	1.88	2.25	2.74	3.40
650	420.1	9.17	1.31	1.40	1.61	1.87	2.19	2.61	3.18	3.96
700	452.4	9.88	1.52	1.61	1.84	2.14	2.51	2.99	3.64	4.52

Disch	arge in		1		Loss of I	Head in I	eet per 10	000 feet o	f length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth c=130	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New.	Steel Pipe 10 Years Old, Brick Sewers. c = 100	Rough.	Very Rough
30	19.39	0.38	0.00	0.004	0.004	0.005	0.006	0.007	0.008	0.01
40	25.85	0.51	0.00	0.006	0.007	0.008	0.010	0.012	0.014	0.01
50	32.32	0.64	0.01	0.009	0.011	0.013	0.015	0.018	0.021	0.02
60	38.78	0.76	0.01	0.013	0.015	0.018	0.021	0.025	0.030	0.03
70	45.24	0.89	0.01	0.018	0.020	0.023	0.027	0.033	0.040	0.05
80	51.7	1.02	0.02	0.022	0.026	0.030	0.035	0.042	0.051	0.06
90	58.2	1.15	0.02	0.028	0.032	0.037	0.044	0.052	0.064	0.07
100	64.6	1.27	0.03	0.034	0.039	0.045	0.053	0.063	0.077	0.09
110	71.1	1.40	0.03	0.041	0.047	0.054	0.064	0.076	0.092	0.11
120	77.5	1.53	0.04	0.048	0.055	0.064	0.075	0.089	0.108	0.13
140	90.5	1.78	0.05	0.064	0.073	0.085	0.100	0.118	0.144	0.17
160	103.4	2.04	0.06	0.082	0.094	0.108	0.127	0.152	0.184	0.22
180	116.3	2.29	0.08	0.102	0.116	0.134	0.158	0.188	0.229	0.28
200	129.3	2.55	0.10	0.123	0.141	0.163	0.192	0.229	0.279	0.34
220	142.2	2.80	0.12	0.147	0.168	0.195	0.229	0.273	0.332	0.41
240	155.1	3.06	0.15	0.172	0.197	0.229	0.269	0.321	0.390	0.48
260	168.0	3.31	0.17	0.200	0.229	0.267	0.312	0.372	0.452	0.56
280	181.0	3.56	0.20	0.228	0.263	0.305	0.359	0.428	0.52	0.65
300	193.9	3.82	0.23	0.260	0.298	0.347	0.407	0.484	0.59	0.74
320	206.8	4.07	0.26	0.293	0.337	0.390	0.459	0.55	0.66	0.83
340	219.7	4.33	0.29	0.328	0.377	0.438	0.51	0.61	0.74	0.92
360	232.7	4.58	0.33	0.364	0.418	0.485	0.57	0.68	0.82	1.03
380	245.6	4.84	0.36	0.402	0.462	0.54	0.63	0.75	0.92	1.14
400	258.5	5.09	0.40	0.442	0.102	0.59	0.69	0.82	1.00	1.25
420	271.5	5.35	0.44	0.484	0.56	0.64	0.76	0.90	1.10	1.37
440	004 4	E 60	0.49	0.53	0.61	0.70	0.83	0.98	1.19	1.49
440	284.4	5.60				0.76	0.83	1.07	1.30	
460	297.3	5.86	0.53	0.57	0.66			1.16	1.42	1.62
480	310.2	6.11	0.58	0.62	$0.71 \\ 0.77$	0.83 0.90	$\begin{array}{c} 0.97 \\ 1.04 \end{array}$	1.10	1.52	1.76
500 550	323.2 355.5	6.37 7.00	0.63	0.80	$0.77 \\ 0.92$	1.07	1.04 1.25	1.48	1.82	1.88 2.26
300	555.5									
600	387.8	7.64	0.91	0.94	1.08	1,.25	1.47	1.75	2.13	2.65
650	420.1	8.27	1.06	1.08	1.25	1.45	1.71	2.04	2.48	3.07
700	452.4	8.91	1.23	1.25	1.43	1.67	1.96	2.33	2.83	3.52
750	484.7	9.55	1.42	1.42	1.63	1.88	2.22	2.64	3.22	4.00
800	517	10.18	1.61	1.59	1.83	2.12	2.49	2.98	3.62	4.50

Disch	arge in				Loss of I	Iead in I	eet per 10	000 feet o	f length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Velocity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth c=130	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New. c=110	Steel Pipe 10 Years Old, Brick Sewers. c=100	Rough.	Very Rough.
30	19.39	0.32	0.00	0.002	0.003	0.003	0.004	0.004	0.005	0.006
40	25.85	0.42	0.00	0.004	0.005	0.005	0.006	0.007	0.009	0.011
50	32.32	0.53	0.00	0.006	0.007	0.008	0.009	0.011	0.013	0.017
60	38.78	0.63	0.01	0.009	0.010	0.011	0.013	0.016	0.019	0.024
80	51.7	0.84	0.01	0.014	0.016	0.019	0.022	0.026	0.032	0.040
100	64.6	1.05	0.02	0.021	0.025	0.028	0.034	0.040	0.048	0.060
120	77.5	1.26	0.02	0.030	0.035	0.040	0.047	0.056	0.068	0.085
140	90.5	1.47	0.03	0.040	0.046	0.054	0.063	0.075	0.091	0.113
160	103.4	1.68	0.04	0.052	0.059	0.068	0.080	0.096	0.117	0.145
180	116.3	1.89	0.06	0.064	0.073	0.085	0.100	0.119	0.144	0.180
200	129.3	2.10	0.07	0.078	0.089	0.103	0.122	0.144	0.176	0.218
220	142.2	2.31	0.08	0.092	0.107	0.123	0.144	0.172	0.208	0.260
240	155.1	2.52	0.10	0.108	0.124	0.144	0.169	0.202	0.246	0.307
260	168.0	2.74	0.12	0.126	0.144	0.167	0.196	0.234	0.285	0.354
2 80	181.0	2.95	0.13	0.144	0.166	0.192	0.226	0.268	0.327	0.407
300	193.9	3.16	0.15	0.164	0.188	0.219	0.257	0.305	0.371	0.462
320	206.8	3.37	0.18	0.184	0.211	0.246	0.289	0.344	0.419	0.52
340	219.7	3.58	0.20	0.207	0.238	0.276	0.322	0.386	0.469	0.58
3 60	232.7	3.79	0.22	0.230	0.262	0.306	0.359	0.429	0.52	0.65
3 80	245.6	4.00	0.25	0.254	0.291	0.339	0.398	0.472	0.58	0.72
400	258.5	4.20	0.27	0.279	0.320	0.372	0.437	0.52	0.63	0.79
420	271.5	4.42	0.30	0.306	0.351	0.407	0.478	0.57	0.69	0.86
440	284.4	4.62	0.33	0.332	0.382	0.442	0.52	0.62	0.76	0.94
460	297.3	4.84	0.36	0.361	0.415	0.481	0.56	0.68	0.82	1.02
480	310.2	5.05	0.40	0.391	0.449	0.52	0.61	0.73	0.89	1.11
500	323.2	5.26	0.43	0.421	0.483	0.56	0.66	0.79	0.96	1.18
550	355.5	5.79	0.52	0.50	0.58	0.67	0.79	0.94	1.14	1.42
600	387.8	6.30	0.62	0.59	0.68	0.78	0.92	1.11	1.34	1.67
650	420.1	6.84	0.73	0.68	0.78	0.92	1.07	1.28	1.56	1.93
700	452.4	7.36	0.84	0.79	0.90	1.05	1.23	1.47	1.78	2.22
750	484.7	7.89	0.97	0.90	1.03	1.18	1.39	1.67	2.03	2.52
800	517	8.42	1.10	1.01	1.16	1.34	1.58	1.88	2.29	2.84
850	549	8.94	1.24	1.13	1.29	1.50	1.77	2.10	2.56	3.19
900	582	9.47	1.39	1.26	1.44	1.67	1.96	2.33	2.84	3.54
950	614	9.99	1.55	1.38	1.59	1.84	2.17	2.59	3.13	3.90

Disch	arge in			:	Loss of H	ead in F	eet per 10	00 feet o	f length.	
Cubic Feet per Second.	Million Gallons per 24 Hours.	Veloc- ity in Feet per Second.	Veloc- ity Head, Feet.	Ex- tremely Smooth and Straight c=140	Very Smooth	Good Ma- sonry Aque- ducts. c=120	Riveted Steel Pipe, New. c=110	Steel Pipe 10 Years Old, Brick Sewers. c=100	Rough.	Very Rough
40	25.85	0.35	0.00	0.003	0.003	0.003	0.004	0.005	0.006	0.00
60	38.78	0.53	0.00	0.005	0.006	0.007	0.009	0.010	0.012	0.01
80	51.7	0.71	0.01	0.009	0.011	0.012	0.014	0.017	0.021	0.020
100	64.6	0.88	0.01	0.014	0.016	0.019	0.022	0.026	0.032	0.040
120	77.5	1.06	0.02	0.020	0.023		0.031	0.037	0.045	0.05
140	90.5	1.24	0.02	0.026	0.030	0.035	0.041	0.049	 0.059	0.07
160	103.4	1.41	0.03	0.034	0.039	0.045	0.052	0.062	0.076	0.09
180	116.3	1.59	0.04	0.042	0.048	0.056	0.065	0.078	0.094	0.11
200	129.3	1.77	0.05	0.050	0.058	0.068	0.079	0.094	0.115	0.14
22 0	142.2	1.94	0.06	0.060	0.070	0.080	0.094	0.113	0.137	0.17
2 40	155.1	2.12	0.07	0.071	0.082	0.094	0.111	0.132	0.161	0.20
260	168.0	2.30	0.08	0.082	0.094	0.109	0.128	0.153	0.186	0.23
2 80	181.0	2.48	0.09	0.094	0.108	0.126	0.148	0.176	0.213	0.26
3 00	193.9	2.65	0.11	0.107	0.123	0.143	0.168	0.200	0.242	0.30
320	206.8	2.83	0.12	0.121	0.139	0.161	0.188	0.226	0.273	0.34
340	219.7	3.01	0.14	0.136	0.156	0.181	0.211	0.252	0.307	0.38
3 60	232.7	3.18	0.16	0.151	0.173	0.200	0.235	0.281	0.341	0.42
380	245.6	3.36	0.18	0.167	0.191	0.222	0.260	0.309	0.377	0.46
400	258.5	3.54	0.19	0.183	0.209	0.243	0.287	0.341	0.414	0.52
420	271.5	3.71	0.21	0.201	0.230	0.267	0.313	0.373	0.455	0.57
440	284.4	3.89	0.23	0.218	0.249	0.290	0.341	0.406		1 .
460	297.3	4.07	0.26	0.237	0.272		0.371	0.441	0.54	0.67
480	310.2	4.24	0.28	0.256	0.293		0.400	0.477	0.58	0.72
500	323.2	4.42	0.30	0.277	0.318	0.369		0.52	0.63	0.78
550	355.5	4.86	0.37	0.330	0.379	0.439	0.52	0.62	0.75	0.98
600	387.8	5.30	0.44	0.388	0.448	0.52	[!] 0.61	0.72	0.88	1.08
	420.1		0.51	0.450	0.52	0.60	0.70	0.84	1.02	1.2
700	452.4	6.19	0.59	0.52	0.59		0.80	0.96	1.17	1.4
750	484.7	6.63	0.68	0.58	0.67	0.78	0.92	1.09	1.33	1.6
800	517	7.07	0.78	0.66	0.76	0.88	1.03	1.23	1.49	1.8
850	549	7.51	0.88	0.74	0.85	0.98	1.16	1.38	1.67	2.0
900	583	7.96	28.0	0.82	0.94	1.09	1.28	1.53	1.86	2.3
950	614	8.40	1.09	0.91	1.04	1.21	1.42	1.69	2.06	2.5
1000	646	8.84	1.21	1,00	1.14	1.33	1.56	1.86	2.27	2.8
1100	711	9.73	1.46	, 1,19	1.37	1.58	1.86	2.22	2.70	3.3

RELATIVE DISCHARGING CAPACITIES OF AQUEDUCTS.



	Relative	Elements Flowin		its when	At Approximate Point of Maximum Discharge.					
	Area.	Wetted Perim- eter.	Mean Hy- draulic Radius.	Velocity.	Area.	Wetted Perim- eter.	Mean Hy- draulic Radius.	Velocity.		
Circle $r_1 = 1.5r$ $r_1 = 2.0r$ $r_1 = 2.5r$ $r_1 = 3r$ $r_1 = 4r$ $\frac{1}{2} \text{ square}$ Square	1000 1034 1057 1071 1078 1089 1136 1273	1000 1023 1040 1054 1063 1076 1136 1273	1000 1011 1018 1018 1016 1014 1000 1000	1000 1007 1011 1011 1010 1009 1000	975 1009 1032 1046 1053 1064 1111 1273	841 864 881 895 904 917 977 955	1160 1168 1172 1169 1165 1160 1137	1098 1103 1106 1104 1101 1098 1083 1199		

AQUEDUCTS,—8 TO 14 FEET.

c=125. At point of maximum discharge the quantity is taken as 12% greater than in a circular aqueduct of the same height and width running full.

Slope in Feet er 1000.	Slope in Feet per Mile.	8′	9′	10′	11'	12′	18′	14'
er 1000.	per Mile.			Discharge i	n Million G	allons Daily	7.	
0.030	0.158	34	46	60	78	98	120	146
0.035	0.185	36	50	66	84	106	130	159
0.040	0.211	39	53	71	91	114	140	171
0.045	0.238	42	57	75	97	121	150	182
0.050	0.264	44	60	79	102	128	158	192
0.055	0.290	46	63	84	108	135	167	203
0.060	0.317	49	66	88	112	142	175	212
0.065	0.343	51	69	91	118	148	182	221
0.070	0.370	53	72	95	122	154	190	231
0.080	0.422	57	78	102	132	166	205	248
0.090	0.475	61	83	109	140	176	218	265
0.10	0.528	64	88	116	148	186	230	280
0.11	0.581	68	92	122	156	196	242	295
0.12	0.634	71	97	127	164	205	254	309
0.14	0.739	77	105	138	178	224	276	336
0.16	0.845	83	113	149	192	240	297	361
0.18	0.950	88	120	159	204	256	316	385
0.20	1.056	93	127	168	215	271	335	407
0.22	1.162	98	134	177	227	285	352	428
0.24	1.267	103	140	185	239	300	370	450
0.26	1.373	108	147	194	249	313	386	469
0.28	1.478	112	153	201	259	325	402	488
0.30	1.584	116	159	209	269	338	418	508
0.35	1.848	126	172	227	291	366	453	550
0.40	2.112	136	185	244	314	395	487	591
0.45	2.376	145	197	260	335	420	519	631
0.50	2.640	153	209	275	354	445	549	668
0.55	2.904	162	219	290	373	468	579	701
0.60	3.168	169	230	304	390	490	606	736
0.65	3.432	177	240	317	407	511	631	770
0 .70	3.696	184	250	330	424	533	659	800
0.80	4.224	197	269	355	456	573	709	860
0.90	4.752	210	287	378	485	610	754	918
1.00	5.28	223	304	400	514	647	800	970
1.10	5.81	235	319	421	541	680	840	1020

AQUEDUCTS,—15 TO 21 FEET.

c=125. At point of maximum discharge the quantity is taken as 12% greater than in a circular aqueduct of the same height and width running full.

Slope in Feet	Slope	15'	16′	17′	18′	19′	20′	21′
in Feet er 1000.	in Feet per Mile.	· · · · · ·	· · · · · · · · · · · · · · · · · · ·	Discharge	in Million G	allons Dail	· · · · · · · · · · · · · · · · · · ·	
				Discharge		anons Dan	y. 1	
0.020	0.106	140	167	196	228	263	300	341
0.022	0.116	148	176	205	239	276	316	358
0.024	0.127	155	184	215	250	289	330	376
0.026	0.137	162	192	227	261	303	346	392
0.028	0.148	169	200	237	274	315	360	410
0.030	0.158	176	208	245	285	326	374	426
0.035	0.185	190	226	266	310	355	406	460
0.040	0.211	205	243	286	330	381	437	495
0.045	0.238	218	258	305	352	406	465	528
0.050	0.264	232	274	323	372	430	493	560
0.055	0.290	243	288	340	395	453	518	588
0.060	0.317	254	300	353	410	475	542	617
0.065	0.343	266	315	372	433	495	569	642
0.070	0.370	277	328	388	450	516	591	670
0.080	0.422	298	353	410	480	552	635	720
0.09	0.475	317	37 6	440	510	591	670	770
0.10	0.528	336	398	470	542	625	718	810
0.11	0.581	354	420	490	570	660	750	860
0.12	0.634	370	439	510	600	6 9 0	790	900
0.14	0.739	404	477	562	650	750	860	980
0.16	0.845	432	512	600	700	810	920	1050
0.18	0.950	461	547	640	740	860	980	1120
0.20	1.056	488	579	680	790	910	1040	1180
0.22	1.162	513	610	710	830	960	1100	1240
0.24	1.267	540	640	750	870	1000	1150	1300
0.26	1.373	562	668	780	910	1050	1200	1360
0.28	1.478	585	694	810	940	1090	1250	1420
0.30	1.584	608	720	840	980	1130	1300	1470
0.35	1.848	660	780	915	1060	1230	1410	1600
0.40	2.112	710	841	990	1140	1320	1520	1720
0.45	2.376	758	896	1050	1220	1410	1620	1830
0.50	2.640	800	950	1110	1290	1490	1700	1940
0.55	2.904	842	1000	1170	1360	1570	1800	2040
0.60	3.168	885	1040	1230	1420	1650	1880	2130
0.65	3.432	921	1090	1280	1480	1720	1960	2230

SEWERS. TABLE OF SLOPES REQUIRED TO PRODUCE GIVEN VELOCITIES. Tile, c=110. Brick, c=100.

C!		Cubic Feet per	v=2	v = 2.5	v=3	v=4	v=5	v=7	v=10
Si	ze.	Second. $v=1$			Slope	in Feet pe	er 1000.		
4"	Tile	0.087	6.5	9.8	13.8	23.5	35.5	66.0	128
5′′	"	0.136	5.0	7.6	10.6	18.1	27.3	51.0	99
6"	"	0.196	4.05	6.1	8.6	14.6	22.0	41.1	80
8"	"	0.349	2.90	4.39	6.2	10.5	15.8	29.5	57
10"	"	0.545	2.24	3.39	4.74	8.1	12.2	22.8	44
12"	"	0.785	1.80	2.73	3.82	6.5	9.8	18.4	35.6
15"	"	1.23	1.39	2.10	2.95	5.0	7.6	14.2	27.5
18"	"	1.77	1.13	1.70	2.38	4.06	6.1	11.5	22.2
21"	"	2.41	0.94	1.42	1.99	3.40	5.1	9.6	18.5
24"	"	3.14	0.80	1.22	1.71	2.90	4.39	8.2	15.9
27"	"	3.98	0.70	1.06	1.49	2.52	3.82	7.1	13.8
30"	"	4.91	0.62	0.94	1.31	2.24	3.39	6.3	12.2
30"	Brick	4.91	0.74	1.12	1.56	2.68	4.04	7.5	14.6
36 ′′	"	7.07	0.60	0.90	1.26	2.16	3.27	6.1	11.8
42′′	"	9.62	0.50	0.76	1.06	1.80	2.72	5.1	9.8
48"	"	12.57	0.428	0.64	0.91	1.54	2.33	4.34	8.4
$54^{\prime\prime}$	"	15.9	0.372	0.56	0.79	1.34	2.03	3.79	7.4
60′′	"	19.6	0.330	0.50	0.70	1.19	1.80	3.35	6.5
$66^{\prime\prime}$	"	23.8	0.295	0.445	0.62	1.06	1.61	3.00	5.8
72′′	"	28.3	0.267	0.402	0.56	0.96	1.45	2.71.	5.3
78"	"	33.2	0.242	0.367	0.52	0.88	1.32	2.47	4.7
84''	"	38.5	0.222	0.336	0.471	0.80	1.21	2.26	4.3
90"	"	44.2	0.205	0.310	0.434	0.74	1.12	2.09	4.0
96′′	"	50.3	0.190	0.288	0.403	0.69	1.04	1.94	3.7
108′′	"	63.6	0.166	0.251	0.372	0.60	0.90	1.69	3.2
10′	"	78.5	0.147	0.221	0.311	0.53	0.80	1.49	2.9
11'	"	95.0	0.131	0.199	0.278	0.472	0.72	1.33	2.5
12'	"	113	0.119	0.179	0.251	0.428	0.65	1.21	2.3
13′	"	133	0.108	0.163	0.229	0.390	0.59	1.10	2.1
14′	"	154	0.099	0.150	0.210	0.358	0.54	1.01	1.9
15'	u	177	0.091	0.138	0.194	0.330	0.50	0.93	1.8
16'	"	201	0.085	0.128	0.180	0.306	0.462	0.86	1.6
17′	"	227	0.079	0.119	0.167	0.285	0.430	0.80	1.5
18′	"	254	0.074	0.111	0.156	0.266	0.403	0.75	1.4
20'	"	314	0.065	0.099	0.138	0.236	0.356	0.66	1.2

TILE SEWERS,—4 TO 12 INCHES.

c = 110.

Slope	4"	5"	6"	8"	10"	12"
in Feet per 1000.		Discharge	in Cubic Feet	per Second, R	unning Full.	
1.8						1.57
2.0						1.66
2.2						1.75
2.4					1.13	1.83
2.6	••••				1.18	1.91
2.8					1.23	1.99
3.0				0.71	1.28	2.06
3.5				0.77	1.39	2.24
4.0			0.39	0.83	1.49	2.41
4.5	••••		0.41	0.88	1.59	2.56
5		0.27	0.44	0.94	1.68	2.72
6		0.30	0.48	1.03	1.86	3.00
7	0.18	0.33	0.53	1.12	2.02	3.26
8	0.19	0.35	0.57	1.20	2.17	3.50
9	0.21	0.37	0.60	1.28	2.31	3.74
10	0.22	0.40	0.64	1.36	2.45	3.95
12	0.24	0.44	0.71	1.50	2.70	4.36
14	0.26	0.47	0.77	1.63	2.94	4.75
16	0.28	0.51	0.82	1.76	3.15	5.1
18	0.30	0.54	0.88	1.87	3.36	5.4
20	0.32	0.58	0.93	1.98	3.56	5.8
22	0.34	0.60	0.98	2.09	3.75	6.1
24	0.35	0.64	1.03	2.19	3.94	6.4
26	0.37	0.66	1.07	2.28	4.10	6.6
28	0.38	0.69	1.11	2.38	4.28	6.9
30	0.40	0.72	1.15	2.46	4.43	7.2
35	0.43	0.78	1.26	2.68	4.83	7.8
40	0.46	0.84	1.35	2.88	5.2	8.4
45	0.49	0.89	1.44	3.07	5.5	8.9
50	0.52	0.94	1.52	3.25	5.8	9.4
60	0.58	1.04	1.68	3.58	6.4	10.4
70	0.63	1.13	1.83	3.90	7.0	11. 3
80	0.67	1.21	1.96	4.18	7.5	12.1
90	0.72	1.30	2.10	4.46	8.0	12.9
100	0.76	1.37	2.22	• 4.73	8.5	13.7

Quantities corresponding to velocities between 2 and 3 and over 10 feet per second are in italics.

TILE SEWERS,—15 TO 36 INCHES. c=110.

Slope	15"	18″	21"	24"	27"	80″	86"
Slope in Feet er 1000.		Disch	arge in Cubic	Feet per Se	cond, Runni	ng Full.	<u> </u>
0.5							14.1
0.6							15.6
0.7					7.9	10.5	16.9
0.8				6.3	8.5	11.3	18.2
0.9	••••			6.7	9.1	12.0	19.4
1.0			5.0	7.1	9.6	12.7	20.5
1.2		3.7	5.5	7.8	10.6	14.0	22.6
1.4	2.5	4.0	6.0	8.5	11.5	15.2	24.6
1.6	2.6	4.3	6.4	9.1	12.4	16.4	26.5
1.8	2.8	4.5	6.8	9.7	13.2	17.4	28.2
2.0	3.0	4.8	7.2	10.3	14.0	18.4	29.8
2.2	3.1	5.1	7.6	10.8	14.7	19.4	31.4
2.4	3 .3	5.3	8.0	11.4	15.4	20.4	32.9
2.6	3.4	5.5	8.3	11.8	16.1	21.2	34.4
2.8	3.6	5.8	8.7	12.3	16.8	22.1	35.7
3.0	3.7	6.0	9.0	12.8	17.4	23 .0	37.1
3.5	4.0	6.5	9.8	13.9	18.9	25.0	40.3
4.0	4.3	7.0	10.5	14.9	20.4	26.9	43.4
4.5	4.6	7.5	11.2	15.9	21.6	28.6	46.2
5.0	4.9	7.9	11.9	16.8	23.0	30.3	48.9
6	5.4	8.7	13.1	18.6	25.4	33.4	54
7	5.9	9.5	14.2	20.2	27.5	36.4	59
8	6.3	10.2	15.3	21.7	29.6	39.0	63
9	6.7	10.9	16.3	23.1	31.5	41.6	67
10	7.1	11.5	17.2	24.5	33.4	44.0	71
12	7.8	12.7	19.0	27.0	36.8	48.6	78
14	8.5	13.8	20.6	29.4	40.0	<i>53</i>	85
16	9.1	14.8	22.2	31.5	43.0	57	92
18	9.7	15.8	23.6	<i>33.6</i>	45.8	60	98
20	10.3	16.7	25.0	<i>35</i> . <i>6</i>	48.5	64	103
22	10.9	17.6	26.4	37.5	51	67	109
24	11.4	18.4	27.6	<i>39.3</i>	53	71	114
26	11.9	19.2	28.9	41.0	56	74	119
28	12.4	20.0	30.0	42.7	58	77	124
30	12.8	2 0.8	31.1	44.2	60	80	128

Quantities corresponding to velocities between 2 and 3 and over 10 feet per second are in italics.

BRICK SEWERS,—30 TO 66 INCHES. c=100.

Slope	80″	86′′	42"	48"	54"	60″	66"
Slope in Feet per 1000.		Discha	rge in Cubic 1	Feet per Sec	ond, Running	g Full.	
0.30							48
0.35			l . [41	52
0.40					33	44.	56
0.45				26	35	46	60
0.50	••••		19.3	27	37	49	63
0.55			20.3	29	39	52	67
0.60		14.2	21.2	<i>30</i>	41	54	70
0.65		14.8	22.2	<i>32</i>	43	57	73
0.70		15.4	23.1	<i>33</i>	45	<i>59</i>	76
0.80	10.2	16.6	24.8	<i>35</i>	48	63	82
0.9	10.9	17.6	26.5	<i>38</i>	51	68	87
1.0	11.6	18.7	28.0	40	54	71	92
1.1	12.2	19.7	29.5	42	57	75	97
1.2	12.8	2 0. 6	30.9	44	60	79	101
1.4	13.9	22.4	33.5	48	65	86	110
1.6	14.9	24.0	36.0	51	70	92	118
1.8	15.9	25.6	38.4	55	74	98	126
2.0	16.8	27.1	40.6	58	79	104	134
2.2	17.7	28.6	42.9	61	83	110	141
2.4	18.5	29.9	44.9	64	87	115	147
2.6	19.3	31.2	46.8	66	91	120	154
2.8	20.1	32.5	48.8	69	94	125	160
3.0	20.9	33.8	51	72	98	130	166
3.5	22.7	36.7	55	78	107	141	181
4.0	24.4	39.5	59	84	114	151	194
4.5	26.0	42.0	63	90	122	161	207
5.0	27.5	44.5	67	95	129	170	219
5.5	29.0	47	70	100	136	180	231
6.0	30.4	49	74	105	143	188	241
6.5	31.8	51	77	109	149	197	253
7	33.0	53	80	114	155	205	263
8	35.5	57	86	122	<i>166</i>	220	282
9	<i>3</i> 7.8	61	92	130	178	23 4	301
10	40.0	65	97	138	188	248	319
11	42.1	<i>68</i>	102	145	198	2 61	335

Quantities corresponding to velocities between 2 and 3 and over 7 feet per second are in italics.

BRICK SEWERS,—72 TO 108 INCHES. c=100.

Slope	72″	78"	84"	90″	96″	108″
Slope in Feet per 1000.		Discharge i	n Cubic Feet 1	per Second, Ru	inning Full.	
0.18						133
0.20	••••	1	••••		103	141
0.22	••••		77	92	109	148
0.24	••••	66	80	97	114	156
0.24		69	84	101	119	162
0.20	••••		-			
0.28	58	72	87	105	124	169
0.30	60	74	91	109	129	175
0.32	62	77	94	113	133	182
0.34	65	80	97	116	138	188
0.36	66	82	100	120	142	194
0.00						
0.38	69	85	103	124	146	199
0.40	71	87	106	127	150	205
0.45	75	93	113	136	160	218
0.50	79	98	119	144	169	230
0.55	84	103	126	151	178	243
0.00						
0.60	88	108	132	158	187	255
0.65	92	113	138	166	196	266
0.70	95	118	143	172	203	277
0.75	99	122	149	179	211	288
0.8	102	126	154	185	218	298
• • •				1		
0.9	109	135	164	197	233	316
1.0	116	143	173	207	246	335
1.1	122	150	182	220	259	353
1.2	128	158	192	230	272	370
1.3	133	164	200	240	284	386
1.4	139	171	208	250	295	402
1.5	144	178	216	260	306	418
1.6	149	184	224	269	317	433
1.8	159	196	238	287	338	460
2.0	168	207	252	304	357	488
0.0	170	010	907	010	000	
2.2	176	218	265	319	376	510
2.4	185	229	278	335	395	540
2.6	194	239	290	349	412	560
2.8	201	249	302	364	429	570
3.0	209	258	314	378	446	610

Quantities corresponding to velocities between 2 and 3 and over 7 feet per second are in italics.

BRICK SEWERS,—10 TO 15 FEET.
c=100.

Slope	10′	11'	12'	18′	14'	15′
in Feet per 1000.		Discharge	in Cubic Feet	per Second, R	unning Full.	1
0.09				1		350
0.10					310	372
0.11				268	326	391
0.12	• • • •	l	228	281	341	410
0.13	••••		238	294	356	428
0.14		197	248	305	371	445
0.15	159	205	257	318	385	462
0.16	165	211	266	329	400	479
0.18	176	225	284	350	425	510
0.20	186	239	300	370	450	540
0.22	196	251	316	390	474	570
0.24	2 05	263	<i>331</i>	409	496	600
0.26	214	275	346	426	520	620
0.28	222	286	360	444	540	650
0.30	231	297	374	461	560	670
0.32	240	307	387	477	580	700
0.34	247	318	400	494	600	720
0.36	255	328	412	510	620	740
0.38	262	337	425	520	640	760
0.40	270	347	436	540	650	780
0.45	288	370	465	570	700	840
0.50	305	391	492	610	740	890
0.55	321	412 .	520	640	780	930
0.60	33 6	432	540	670	810	980
0.65	351	451	570	700	850	1020
0.70	365	470	590	730	890	1060
0.75	380	488	610	760	920	1100
0.8	392	500	630	780	950	1140
0.9	418	540	680	830	1010	1220
1.0	443	570	720	880	1070	1290
1.1	466	600	750	930	1130	· 1360
1.2	488	630	790	980	1180	1420
1.3	510	660	<i>820</i>	1020	1240	1480
1.4	530	680	860	1060	1290	1540
1.5	<i>550</i>	710	<i>890</i>	1100	134 0	1600

Quantities corresponding to velocities between 2 and 3 and over 7 feet per second are in italics.

COMPUTATION OF DECREASE IN THE VALUE OF c IN CAST-IRON PIPE, WITH AVERAGE SOFT UNFILTERED RIVER WATER, THROUGH A PERIOD OF YEARS.

1st. Assume that the original value of c is 130.

2d. Assume that the increase in loss of head due to tuberculation, etc., amounts to 3% per year.

3d. Assume that the diameter of the pipe is reduced by tuberculation at the rate of 0.01 inch per year, and that the value of c must be modified to correct for this.

Age of Pipe in	Value of c, with no Al- lowance for	4"	6''	8"	10"	12"	16"	20"	24"	80″	86"	48"	60"
Years.	Reduction in Diameter.		Value	of ca	fter M	aking	Allow	ance f	or Dec	rease	in Dis	meter	. '
0	130	130	130	130	130	130	130	130	130	130	130	130	130
10	113	106	108	109	110	110	111	111	112	112	112	112	112
20	101	88	92	94	96	97	98	99	99	99	99	100	100
30	92	75	80	83	85	86	87	88	89	90	90	90	91
40	85	64	71	74	76	78	79	80	81	82	83	83	84
50	79.3	56	63	67	69	71	73	74	75	76	76	77	78
60	74.6	48	56	61	63	65	67	69	70	71	71	72	73
70	70.6	42	51	55	58	60	62	64	65	66	67	67	68
80	67.1	37	46	51	54	56	58	60	61	62	63	64	65
90	64.2	33	42	47	50	52	55	57	58	59	60	61	62
100	61.5	29	38	43	47	49	52	54	55	56	57	58	59

COMPARISON OF THE LOSS OF HEAD OF WATER IN PIPES OF VARIOUS AGES, AS COMPUTED BY THE METHODS USED

- (1) by Coffin: "Graphical Solution of Hydraulic Problems."
- (2) by Weston: "Friction of Water in Pipes."
- (3) by HAZEN & WILLIAMS: Figures used in this volume.

Age of	Diam-	1 Foo	Velocity of t per Se	of cond.	3 Fe	Velocity of et per Se	of cond.	Velocity of 5 Feet per Second.			
Pipe in Years.	eter of Pipe in Inches.	Coffin.	Weston	Hazen & Wil- liams.	Coffin.	Weston	Hazen & Wil- liams.	Coffin.	Weston	Hazen & Wil- liams.	
New	4	1.55	1.18	1.32	11.7	10.4	10.2	30.0	29.0	26.0	
"	16	0.28	0.25	0.26	2.09	2.20	2.00	5.3	6.2	5.2	
"	48	0.067	0.080	0.072	0.51	0.71	0.55	1.3	2.0	1.4	
10	4	1.88	1.54	1.90	16.0	13.6	15.0	44.0	38.0	38.0	
"	16	0.34	0.33	0.35	2.9	2.9	2.7	7.8	8.1	7.0	
"	48	0.08	0.10	0.10	0.7	0.9	0.7	1.9	2.6	1.9	
20	4	2.30	1.90	2.70	21.0	17.0	21.0	61.0	47.0	53.0	
"	16	0.41	0.41	0.44	3.8	3.6	3.4	11.0	10.0	9.0	
"	48	0.10	0.13	0.12	0.9	1.2	0.9	2.6	3.2	2.3	
40	4	3.10	2.60	4.90	31.0	23.0	38.0	96.0	65.0	96.0	
"	16	0.55	0.56	0.65	5.6	5.0	5.0	17.0	14.0	13.0	
"	48	0.13	0.18	0.17	1.4	1.6	1.3	4.2	4.4	3.3	

SHORT METRIC EQUIVALENT PIPE TABLE.

Discharge in				Loss o	f Head i	Meters	per 100	00 mete	rs of le	ngth.	
Gallons Daily.	Cubic M Dai	leters ly.			•	Diamete	rs in Me	eters.			
c=100 Old.	c=100 Old.	c=130 New.	D=0.1 =3.94 Ins.	D=0.2 =7.87 Ins.	D=0.3 =11.81 Ins.	D=0.4 =15.75 Ins.	D=0.5 =19.68 Ins.	D=0.6 =23.62 Ins.	D=0.8 =31.50 Ins.	D=1.0 =39.37 Ins.	D=1.2 =47.24 Ins.
26,417	100	130	0.6	0.02							
39,626	150	195	1.2	0.04						ŀ	
52,834	200	260	2.0	0.07	0.01				ĺ		
66,042	250	325	3.1	0.11	0.01						}
79,251	300	390	4.3	0.15	0.02						
92,459	3 50	455	5.8	0.20	0.03						
105,668	400	520	7.4	0.25	0.03	0.01		1			
132,085	500	650	11.2	0.38	0.05	0.01				1	ļ
158,502	600	780	15.6	0.54	0.07	0.02	0.01	1	į		
211,336	800	1,040	26.6	0.91	0.13	0.03	0.01				
264,170	1,000	1,300	40.5	1.38	0.19	0.05	0.02	0.01			
317,004	1,200	1,560	57	1.93	0.27	0.07	0.02	0.01			
369,838	1,400	1,820	76	2.58	0.36	0.09	0.03	0.01			
422,672	1,600	2,080	97	3.30	0.46	0.11	0.04	0.02			
475,506	1,800	2,340	120	4.10	0.57	0.14	0.05	0.02			
528,340	2,000	2,600	146	5.0	0.69	0.17	0.06	0.02			
660,425	2,500	3,250	220	7.5	1.05	0.26	0.09	0.04			ļ
792,510	3,000	3,900	310	10.6	1.47	0.36	0.12	0.05	0.01	İ	
1,056,680	4,000	5,200	515	18.0	2.50	0.62	0.21	0.09	0.02	0.01	
1,320,850	5,000	6,500	800	27.2	3.80	0.93	0.31	0.13	0.03	0.01	
1,585,020	6,000	7,800		38	5.3	1.31	0.44	0.18	0.04	0.02	0.01
2,113,360	8,000	10,400		65	9.1	2.23	0.75	0.31	0.08	0.03	0.01
2,641,700	10,000	13,000		99	13.7	3.38	1.13	0.47	0.12	0.04	0.02
3,170,040	12,000	15,600		138	19.2	4.70	1.60	0.65	0.16	0.05	0.02
3,698,380	14,000	18,200		183	25 .6	6.3	2.10	0.87	0.22	0.07	0.03
4,226,720	16,000	20,800		235	32.8	8.0	2.70	1.12	0.28	0.09	0.04
4,755,060	18,000	23,400		292	41.8	10.0	3.40	1.38	0.34	0.12	0.05
5,283,400	20,000	26,000		356	50	12.2	4.10	1.68	0.42	0.14	0.06
6,604,250	25,000	32,500			75	18.4	6.2	2.55	0.63	0.21	0.09
7,925,100	30,000	39,000			105	25.8	8.7	3.55	0.88	0.29	0.12
10,566,800	40,000	52,000	 		180	43	14.8	6.1	1.50	0.50	0.21
13,208,500	50,000	65,000	 		272	67	22.4	9.2	2.26	0.76	0.31
15,850,200	60,000	78,000				93	31.5	12.8	3.20	1.07	0.44
21,133,600		104,000	l .			160	53	22.0	5.4	1.80	0.75
26,417,000			1	l	l	240	81	33.0	8.2	2.73	1.13

VENTURI METERS.

TABLE SHOWING HEAD LOST IN EXCESS OF THAT LOST IN STRAIGHT PIPE, EXPRESSED IN TERMS OF THE VELOCITY HEAD IN THE PIPE.

Note.—The velocity head for any given discharge and pipe size may be found in the pipe tables.

							Diame	eter of	Pipe.	-		•			
Diameter of Throat, Inches.	10"	12"	16"	20"	24"	30"	36"	42"	48′′	54"	60"	66"	72"	78"	84"
111011001				•	Loss	f Hea	d in T	erms c	of Velo	city I	lead.				
4	6	12	39										1		
4.5	4	7	20							ŀ					
5	2	5	15	38											
5.5 6		3 2	10 7	25 18	37										
U		_	' ˈ	10	31										
6.5		l	5	13	26										
7	 		4	10	20										
7.5			3	7	15	36									
8			2	5	11	28									
8.5				4	9	22		İ					,		
9		ļ		3	7	17	35								
9.5				3	6	14	28								
10				2	5	11	23								
11		 			3	7	15	29	,						
12					2	5	11	20	34						
13						4	8	15	25						
14						3	6	11	18	29					
15						2	4	8	14	22	34				
16				<i>.</i>			3	6	11	17	26				
17							3	5	. 8	13	20	29			
18		 					2	4	6	10	16	23	33		
19								3	5	8	13	18	26		
20								2	4	'7	10	15	21	29	
21								2	3	6	8	12	18	24	32
22								• • • •	3	5	7	10	14	20	27
23									2	4	6	8	12	16	22
24									2	3	5	7	10	14	19
25										3	4	6	9	12	16
2 6										2	4	5	. 4	10	14
27										2	3	4	6	9	12
28											3	4	5	7	10
29						[]					2	3	5	6	9
30											2	3	4	6	8
31												3	4	5	7
32		l				اا			!		l	2	3	4	6

UNDERDRAINS FOR SAND FILTERS.

(No compensating orifices used.)

						_	
Rate of filtration, mil-		i	1				
lion gallons per acre						1	
daily	3	4	5	6	8	10	15
Assumed resistance of							
clean sand, feet	0.090	0.120	0.150	0.180	0.240	0.300	0.450
Total allowable friction							
and velocity head in							
underdrainage system	0.022	0.030	0.037	0.045.	0.060	0.075	0.112
Approximate ratio of							
filter area to area of							
main drain	6,300	5,600	5,100	4,700	4,200	3,800	3,200
Approximate velocity in							
main drain (varying							
somewhat with size).	0.67	0.80	0.90	1.00	1.18	1.34	1.68
Approximate velocity							
in laterals (varying							
somewhat with size).	0.40	0.48	0.55	0.61	0.72	0.82	1.04

MAXIMUM AREAS DRAINED IN SQUARE FEET.

				1	1	1	1	1	1	
2"	round	later	ral	79	70	64	59	53	48	41
3′′	"	"		180	160	147	137	122	111	93
4"	"	"		325	.288	264	245	218	200	168
5′′	"	"		517	460	420	390	345	316	266
6"	,"	"	• • • • •	750	670	610	570	500	460	390
.8"	"	"		1,340	1,200	1,090	1,010	900	820	690
6"	split	"		360	320	290	270	240	220	180
8"	"	"		640	570	520	490	430	400	320
10"	"	"		1,020	900	830	770	680	630	530
$12^{\prime\prime}$	"	"	• • • • • •	1,480	1,320	1,200	1,120	1,000	910	770
10"	round	main	1	3,400	3,000	2,700	2,500	2,200	2,000	1,700
12"	"	"		4,900	4,300	3,900	3,600	3,200	2,900	2,400
15"	"	cc	,	7,700	6,900	6,200	5,800	5,100	4,600	3,900
18"	"	"		11,200	10,000	9,000	8,300	7,400	6,700	5,600
21″	"	"		15,300	13,600	12,300	11,400	10,000	9,100	7,600
24"		"		20,000	17,700	16,100	14,900	13,200	12,000	10,000
27"	"	"		25,400	22,400	20,300	18,800	16,600	15,100	12,600
30"	"	"		31,500	27,800	25,300	23,400	20,700	18,800	15,700
33′′	"	"		38,000	34,000	31,000	28,000	25,000	23,000	19,000
36 ′′	"	"		45,000	40,000	37,000	34,000	30,000	27,000	22,000

Note.—For main drains, c is taken as 110, and it is assumed that the space drained is twice as long as wide. For lateral drains, c is taken as 100, and it is assumed that the space drained is four times as long as wide. Considerable change in shape of area drained does not greatly affect the results, and the figures may be used as approximations for all ordinary conditions.

THE FLOW OF WATER OVER WEIRS.

SHARP-EDGED WEIRS.

THE basis of our experimental knowledge of the discharge of water over weirs of size applicable to the cases usually encountered in practice rests primarily upon three investigations, viz.:

- (a) That of Mr. Jas. B. Francis, M. Am. Soc. C. E., made at Lowell, Mass., in 1852.
- (b) That of Messrs. Alphonse Fteley and Frederic P. Stearns, Members Am. Soc. C. E., made at Boston, Mass., in 1877, 1878, and 1879.
- (c) That of M. Henry Bazin, Inspecteur General des Ponts et Chaussees, made at Dijon, France, in 1886, 1887, and 1888.

Each of these investigations has given rise to a formula for determining the flow of water over a sharp-edged vertical weir without end contractions, named from the observers, and these three formulas comprise those most commonly applied in practice.

The symbols used in these formulas and in the following tables are:

- H=the total head or height from the crest of the weir to still water, measured in feet;
- h=the observed head or height of the surface of the running water above the crest of the weir, at some convenient point, measured in feet;
- h_v =the head to which the mean velocity of the approaching water is due, measured in feet—i.e., $h_v = \frac{v^2}{2g}$ —where v=velocity in feet per second;
- L=the total length of the crest of the weir, or the mean width of the over-falling sheet at the plane of the weir, measured in feet;
- p=the height of the crest of the weir above the bottom of the channel of approach, measured in feet;
- Q=the quantity of water discharged per second over a weir, measured in cubic feet;
- g = the acceleration due to gravity = 32.16 feet per second.

The Francis formula, then, is:

$$Q=3.33LH^{\frac{3}{2}}$$
 or $Q=3.33L[(h+h_v)^{\frac{3}{2}}-h_v^{\frac{3}{2}}]$.

The Fteley and Stearns formula is:

$$Q=3.31LH^{\frac{3}{2}}+0.007L$$
 or $Q=3.31L(h+1.5h_v)^{\frac{3}{2}}+0.007L$.

The Bazin formula is:

$$Q = mLh\sqrt{2gh}$$
, where $m = \left(0.405 + \frac{0.00984}{h}\right) \left[1 + 0.55\left(\frac{h}{p+h}\right)^2\right]$.

The several observers used different methods of reading the head h, and for an accurate application of the formulas the head should be read in the same manner as in the original experiments.

Mr. Francis, in the experiments upon which his formula is based, observed the head as communicated through a small orifice (about $\frac{1}{4}$ inch diameter) in the side of the channel of approach, about 1 foot below the level of the crest and 6 feet up-stream therefrom, which was connected through a pipe about 18 inches long to a cistern, where the surface was read by a hook gage. The weir was of L=10 feet.

In a part of their experiments, which were made on a weir with $L\!=\!5$ feet, Messrs. Fteley and Stearns made use of a small orifice in the center of a plank 10 inches long, set with its face vertical and parallel to the axis of the channel of approach, and about 16 inches from the side wall, so that the orifice was about 10 inches above the bottom and 6 feet up-stream from the weir, the orifice being connected by piping to a movable cistern, in which the head was read by a hook gage. For the rest of their experiments these observers made use of eight small orifices simultaneously, which were connected in pairs, opening in opposite directions. These orifices were in the center of steel plates about 6 inches long, located parallel to the current at about the level of the crest of the weir, and were 6 feet up-stream therefrom, and 18 inches and 7 feet respectively from the side walls of the channel, the weir being of $L\!=\!19$ feet.

In the experiments of M. Bazin, who worked on weirs of $L\!=\!6.56$ feet, 3.28 feet, and 1.64 feet, the head was communicated through an orifice 4 inches in diameter, at the bottom of the channel of approach and 16.3 feet up-stream from the weir, connecting with a pit, wherein the surface of water was located by a hook gage and a dial-float.

Experimental comparisons of these formulas, where the heads were observed in the manner described for each, has shown them to agree

within 2½ per cent for heads from 0.5 up to 3 feet, and that the Fteley and Stearns and the Bazin formulas agree within 2 per cent for heads up to 4 feet. The Francis formula was only intended to apply between heads of 0.5 and 2.0 feet, and should not be used for higher heads. Where other methods of reading the head are used, errors of as much as 10 per cent may be introduced. One of the most erroneous of these is by the aid of a pipe placed in the current parallel to the weir and perforated upon its bottom or top.

A very convenient as well as accurate means of reading the head upon a weir, and one which introduces but a small error, is by the use of a sharp-pointed plumb-bob suspended upon a steel tape, the latter passing over a block on which a line is drawn at right angles to the tape, the reading taken being that of the tape where the line intersects it. The reading of the tape corresponding to the position of the bob when in contact with the water surface, when the latter is at the level of the crest of the weir, must be determined and used as the datum. The point of observation should be far enough away from the crest of the weir to be beyond the curve of the approaching sheet, and the elevation of the water surface may be read by allowing the point of the bob to come in contact with it, the bob being still, or by swinging the bob and allowing it to cut the water surface. Whichever method is adopted should be used in determining the datum reading, as the indications are somewhat different. Such readings will be found to fit the Bazin formula more accurately than they will either of the others.

To facilitate the use of this formula, the following table giving the discharge over weirs of various heights from 2 to 30 feet and for heads from 0.1 to 6.0 feet is presented. The discharges in this table can only be used in cases where the level of the water surface on the down-stream side of the weir is below the crest, and the space between the face of the weir and the over-falling sheet is in free connection with the outside air. If a partial vacuum be formed behind the sheet, from lack of free circulation, the discharge will be increased, under some conditions as much as 5 per cent. If the water on the down-stream side rise above the crest, the weir then becomes submerged or drowned and the discharge is consequently decreased.

DISCHARGE PER FOOT OF LENGTH OVER SHARP-EDGED VERTICAL WEIRS, WITHOUT END CONTRACTIONS.

COMPUTED BY BAZIN'S FORMULA.

$$Q = \left(0.405 + \frac{.00984}{h}\right) \left[1 + 0.55 \frac{h^2}{(p+h)^2}\right] Lh\sqrt{2gh}.$$

Observed head =h. Height of weir =p. Discharge =Q. g=32.17 feet. Length of weir =L.

			Tiene	th of wen				
	p=2 Ft.	p=3 Ft.	p=4 Ft.	p=5 Ft.	p=6 Ft.	p=7 Ft.	p=8 Ft.	
h in Feet.	0	O	0	O	0	0	0	h in Feet.
	Q Cu. Ft.	Q Cu. Ft.	Q Cu. Ft.	Q Cu. Ft.	Q Cu. Ft.	Q Cu. Ft.	Q Cu. Ft.	
	per Sec.	per Sec.	per Sec.	per Sec.	per Sec.	per Sec.	per Sec.	
0.1	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.1
0.2	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.2
0.3	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.3
0.4	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.4
0.5	1.23	1.22	1.21	1.21	1.21	1.21	1.21	0.5
0.6	1.62	1.59	1.59	1.58	1.58	1.58	1.58	0.6
0.7	2.04	2.01	1.99	1.98	1.98	1.98	1.98	0.7
0.8	2.50	2.45	2.43	2.42	2.41	2.41	2.41	0.8
0.9	3.00	2.93	2.90	2.88	2.88	2.87	2.86	0.9
1.0	3.53	3.44	3.40	3.38	3.36	3.36	3.35	1.0
1.2	4.68	4.55	4.48	4.47	4.42	4.41	4.40	1.2
1.4	5.99	√5.78	5.68	5.62	5.58	5.56	5.54	1.4
1.5	6.65	6.44	6.30	6.23	6.20	6.18	6.16	1.5
1.6	7.40	7.12	6.97	6.89	6.84	6.80	6.78	1.6
1.8	8.93	8.56	8.37	8.25	8.18	8.13	8.09	1.8
2.0	10.58	10.12	9.87	9.72	9.62	9.55	9.51	2.0
2.2	12.34	11.77	11.46	11.27	11.14	11.06	10.99	2.2
2.4	14.20	13.53	13.15	12.91	12.75	12.64	12.56	2.4
2.5	15.20	14.43	14.09	13.80	13.61	13.50	13.42	2.5
2.6	16.16	15.38	14.92	14.63	14.44	14.30	14.20	2.6
2.8	18.23	17.32	16.79	16.44	16.21	16.04	15.92	2.8
3.0	20.39	19.36	18.74	18.33	18.06	17.86	17.71	3.0
3.2	22.64	21.48	20.77	20.31	19.98	19.75	19.58	3.2
3.4	24.98	23.70	22.89	22.36	21.99	21.72	21.52	3.4
3.5	26.20	24.83	24.00	23.43	23.01	22.73	22.48	3.5
3.6	27.41	25.99	25.09	24.49	24.06	23.75	23.52	3.6
3 .8	29.94	28.38	27.38	26.70	26.22	25.87	25.60	3.8
4.0	32.54	30.84	29.74	28.99	28.45	28.05	27.74	4.0
4.2	35.22	33.39	32.18	31.35	30.75	30.30	29.96	4.2
4.4	37.99	3 6.01	34.70	33.78	33.12	32.62	32.24	4.4
4.6	40.83	38.71	37.29	36.29	3 5.56	35.01	34.58	4.6
4.8	43.75	41.49	39.9 6	3 8.87	3 8.07	37.46	37.00	4.8
5.0	46.71	44.31	42.67	41.49	40.62	39.96	39.44	5.0
5.2	49.81	47.27	45.50	44.23	43.29	42.57	42.01	5.2
5.4	52.94	50.23	48.38	47.02	46.00	45.22	44.60	5.4
5.6	56.15	53.33	51.34	49.88	48.79	47.94	47.28	5.6
5.8	59.42	56.45	54.34	52.79	51.62	50.71	49.99	5.8
6.0	62.77	59.65	57.43	55.78	54 . 53	53.55	52.78	6.0
				66				

DISCHARGE PER FOOT OF LENGTH OVER SHARP-EDGED VERTICAL WEIRS, WITHOUT END CONTRACTIONS.

COMPUTED BY BAZIN'S FORMULA.

$$Q = \left(0.405 + \frac{.00984}{h}\right) \left[1 + 0.55 \frac{h^2}{(p+h)^2}\right] Lh\sqrt{2gh}.$$

Observed head =h. Height of weir =p. Discharge =Q. g=32.17 feet. Length of weir =L.

			Leng	th of weir	<i>-и</i> .			
	p=9 Ft.	p=10 Ft.	p=12 Ft.	p=16 Ft.	p=20 Ft.	p=25 Ft.	p=30 Ft.	,
in Feet.	Q Cu. Ft. per Sec.	in Feet.						
0.1	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.1
0.2	0.33	0.33	0.33	0.33	0.33	0.33	0.33	0.2
0.3	0.58	0.58	0.58	0.58	0.58	0.58	0.58	0.3
0.4	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.4
0.5	1.21	1.21	1.21	1.21	1.20	1.20	1.20	0.5
0.6	1.57	1.57	1.57	1.57	1.57	1.57	1.57	0.6
0.7	1.97	1.97	1.97	1.97	1.97	1.97	1.97	0.7
0.8	2.40	2.40	2.40	2.40	2.40	2.40	2.40	0.8
0.9	2.86	2.86	2.86	2.85	2.85	2.85	2.85	0.9
1.0	3.35	3.34	3.34	3.33	3.33	3.33	3.33	1.0
1.2	4.39	4.38	4.38	4.37	4.36	4.36	4.36	1.2
1.4	5.53	5.52	5.51	5.49	5.49	5.48	5.48	1.4
1.5	6.14	6.13	6.12	6.11	6.10	6.09	6.09	1.5
1.6	6.76	6.74	6.73	6.71	6.69	6.69	6.68	1.6
1.8	8.07	8.05	8.02	7.99	7.98	7.97	7.96	1.8
2.0	9.47	9.44	9.40	9.36	9.34	9.33	9.32	2.0
2.2	10.95	10.91	10.86	10.81	10.78	10.76	10.75	2.2
2.4	12.50	12.45	12.39	12.32	12.28	12.25	12.24	2.4
2.5	13.36	13.28	13.19	13.14	13.10	13.07	13.05	2, 5
2.6	14.13	14.07	13.99	13.90	13.85	13.82	13.80	2.6
2.8	15.83	15.76	15.66	15.54	15.48	15.44	15.42	2.8
3.0	17.60	17.52	17.39	17.25	17.18	17.13	17.10	3.0
3.2	19.45	19.34	19.19	19.02	18.93	18.87	18.83	3.2
3.4	21.36	21.24	21.06	20.86	20.75	20.68	20.63	3.4
3.5	22.38	22.22	22.00	21.83	21.71	21.62	21.60	3.5
3.6	23.34	23.20	22.99	22.75	22.62	22.53	22.48	3.6
3.8	25.39	25.23	24.99	24.71	24.56	24.45	24.39	3.8
4.0	27.51	27.32	27.05	26.72	26.55	26.42	26.35	4.0
4.2	29.69	29.48	29.17	28.79	28.59	28.45	28.36	4.2
4.4	31.94	31.70	31.34	30.92	30.66	30.52	30.42	4.4
4.6	34.25	33 .98	33.58	33.10	32.84	32.65	32.53	4.6
4.8	36.62	36.33	35.88	35.35	35.05	34.83	34.70	4.8
5.0	39.03	38.70	38.21	37.61	37.28	37.03	36.88	5.0
5.2	41.56	41.20	40.65	39.98	39.61	39.33	39.17	5.2
5.4	44.11	43.71	43.12	42.38	41.96	41.66	41.47	5.4
5.6	46.74	46.31	45.65	44.84	44.38	44.04	43.83	5.6
5.8	49.41	48.94	48.22	47.33	46.83	46.45	46.22	5.8
6.0	52 .15	51.64	50.86	49.90	49.34	48.92	48.67	6.0

LOW HEADS.

For heads below 0.2 foot the Bazin Formula gives discharges somewhat in excess of the experimental results of Fteley and Stearns, and in practice accurate weir measurement at low heads becomes extremely difficult on account of the increased relative importance of errors of observation, and of changes in the character of the flow if the edge of the weir has a measurable thickness. It may also be expected that the temperature of the water will exercise considerable influence. For these low heads the formula deduced by Fteley and Stearns for their small weir, Q=3.33LH%+0.0065L, gives results varying from the experiments by from 4 to 6 per cent for heads from 0.2 to 0.07 foot, the lowest observed. The actual results were usually greater than those given by the formula. For a head of 0.1 foot this formula gives a discharge of 0.11 cu. ft. per second, as compared with 0.13 cu. ft. by Bazin. A value of 0.115 cu. ft. seems quite nearly correct for this head.

END CONTRACTIONS.

For weirs having end contractions the formula of Mr. Francis, modified as he proposed by subtracting the quantity 0.1nH from the value of L, making the formula $Q=3.33(L-0.1nH)H^{\frac{6}{2}}$, is the one generally recognized. In this modification n is the number of end contractions, or the proportion of a complete contraction. Recent experiments indicate that the effect of end contractions is not to be provided for by so simple a formula, and until more data are available such weirs should be avoided so far as circumstances will permit.

VERY HIGH WEIRS.

When the weir is of such dimensions in proportion to the channel of approach that the velocity of the approaching water may become zero, the formula of Bazin reduces to $Q = \left(0.405 + \frac{0.00984}{h}\right) Lh\sqrt{2gh}$, which corresponds to p = infinity, and the following table gives the value of the several factors, and the discharge under this condition for L = 1 foot. In this and the preceding table g has been taken as 32.173 feet, that being its value for latitude 40° and an elevation above sea-level of 500 feet.

VALUES OF FACTORS IN BAZIN'S FORMULA AND DISCHARGE OVER WEIR OF INFINITE HEIGHT.

Head = h in Feet.	$\sqrt{2gh}$	$h\sqrt{2gh}$	$\left(0.405 + \frac{0.00984}{h}\right)$	Discharge Q in Cu. Ft. per Sec. for $L=1$ Foot.
0.1	2.537	0.254	0.503	0.13
0.2	3.587	0.717	0.454	0.33
0.3	4.394	1.318	0.438	0.58
0.4	5.073	2.029	0.430	0.87
0.5	5.672	2.836	0.425	1.20
0.6	6.213	3.728	0.421	1.57
0.7	6.711	4.698	0.419	1.97
0.8	7.175	5.740	0.417	2.40
0.9	7.610	6.849	0.416	2.85
1.0	8.021	8.021	0.415	3.33
1.2	8.787	10.544	0.413	4.36
1.4	9.491	13.287	0.412	5.48
1.5	9.824	14.736	0.412	6.07
1.6	10.147	16.234	0.411	6.68
1.8	10.762	19.361	0.410	7.95
2.0	11.344	22.688	0.410	9.30
2.2	11.898	26.178	0.409	10.72
2.4	12.427	29.825	0.409	12.20
2.5	12.683	31.707	0.409	12.97
2.6	12.934	3 3.631	0.409	13.75
2.8	13.423	37.585	0.409	15.35
3.0	13.894	41.682	0.408	17.02
3.2	14.349	45.915	0.408	18.74
3.4	14.791	50.290	0.408	20.51
3.5	15.008	52.523	0.408	21.42
3.6	15.219	54.785	0.408	22.34
3.8	15.637	59.420	0.408	24.22
4.0	16.0 43	64.170	0.407	26.15
4.2	16.439	69.045	0.407	28.13
4.4	16.8 26	74.030	0.407	30.15
4.6	17.204	79.140	0.407	32.22
4.8	17.574	84.360	0.407	34.34
5.0	17.936	89.625	0.407	36.48
5.2	18.292	95.120	0.407	38.70
5.4	18.640	100.656	0.407	40.95
5.6	18.983	106.305	0.407	43.24
5.8	19.318	112.044	0.407	45.56
6.0	19.648	117.888	0.407	47.94

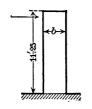
FLAT-CREST AND OTHER WEIRS.

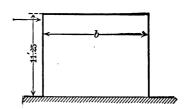
The formulas for the discharge of vertical sharp-edged weirs cease to be applicable when the crest is widened or the up-stream face inclined, and in order to determine what modifications should be made in the computed results, experiments have been made upon some twenty-five models of different forms, with $L\!=\!16$ feet and p as great as 11.25 feet, using heads up to and in some cases a little above 4 feet.

From these experiments the factors by which to multiply the computed discharge for a sharp-edged weir of the same L and p, to give the actual discharge over each form of crest, have been deduced for the heads given in the following tables, wherein the first column gives the head and the columns headed II the multipliers. To use the tables. the discharge for the weir of given form should be first computed as for a vertical sharp-edged weir of the same height and length, using any of the above formulas, or the tables on pages 66, 67, and 69, and the resulting Qs should then be multiplied by the factor in the proper column under II, when the accuracy of the result may be expected to correspond to that of the first computation. So long as the top of the weir is flat and the up-stream face vertical, it appears that the factors given should be applicable to any height of weir, but if the up-stream face or any part of the profile up-stream, from the highest point of the weir, is inclined, the factor will change with the height of the weir, as is shown by the table for triangular weirs.

On all the models having vertical down-stream faces, including model P, air was admitted to the space underneath the sheet. On models D and E experiments were made with the space underneath the sheet unaerated, so that a partial vacuum existed there, which is shown to increase the discharge about 5 per cent at the high heads. For the weirs with inclined down-stream faces, models F to O inclusive, no air was admitted under the sheet. A comparison of the results upon models G and H shows the effect of rounding the up-stream corner of this weir to be an increase in discharge of about 4 per cent at the high heads.

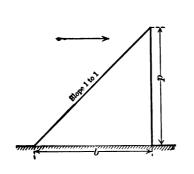
RECTANGULAR FLAT-TOPPED WEIRS.





I. Head	II Multipliers of Discharge over Sharp-edged Vertical Weir of Same L and p .										
in Feet,	b= 0.48 Ft.	b= 0.93 Ft.	b= 1.65 Ft.	b= 3.17 Ft.	b= 5.84 Ft.	b= 8.98 Ft.	b= 12.24 Ft.	b= 16.30 Ft.			
0.5	0.902	0.830	0.819	0.797	0.785	0.783	0.783	0.783			
1.0	0.972	0.904	0.879	0.812	0.800	0.798	0.795	0.792			
1.5	1.000	0.957	0.910	0.821	0.807	0.803	0.802	0.797			
2.0	1.000	0.989	0.925	0.821	0.805	0.800	0.798	0.795			
2.5	1.000	1.000	0.932	0.816	0.800	0.795	0.792	0.789			
3.0	1.000	1.000	0.938	0.813	0.796	0.791	0.787	0.784			
3.5	1.000	1.000	0.942	0.810	0.793	0.787	0.783	0.780			
4.0	1.000	1.000	0.947	0.808	0.790	0.783	0.780	0.777			

TRIANGULAR WEIRS.



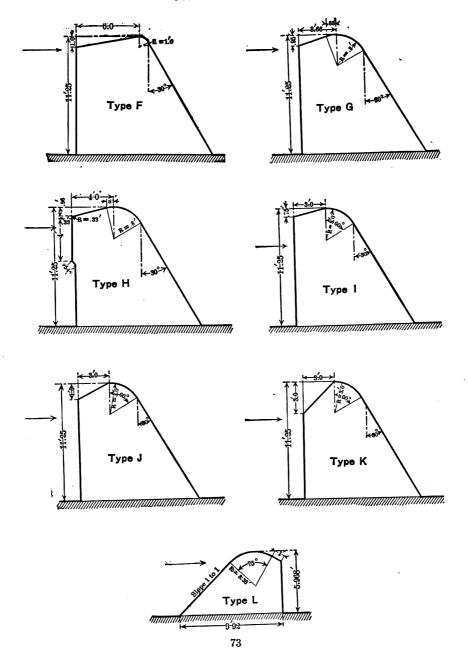
I. Head	II. Mulitpliers.			
in Feet,	b=p= 6.65 Ft.	b=p= 11.25 Ft.		
0.5	1.060	1.060		
1.0	1.079	1.079		
1.5	1.091	1.092		
2.0	1.086	1.097		
2.5	1.076	1.096		
3.0	1.067	1.095		
3.5	1.060	1.094		
4.0	1.054	1.093		

COMPOUND WEIRS.

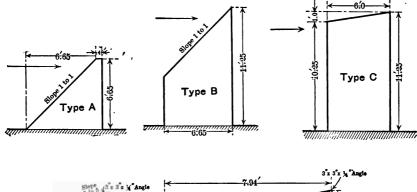
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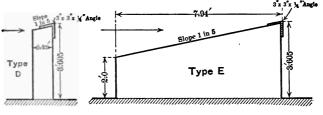
I. Head				II. Multipliers.			
in Feet,	Type F.	Туре G.	Туре Н.	Туре І.	Туре Ј.	Type K.	Туре L.
0.5	0.964	0.932	0.934	0.968	0.971	0.971	0.971
1.0	1.026	0.982	1.000	1.008	1.040	1.040	0.983
1.5	1.064	1.015	1.040	1.032	1.083	1.092	1.012
2.0	1.066	1.031	1.061	1.041	1.105	1.126	1.040
2.5	1.025	1.038	1.073	1.043	1.118	1.146	1.057
3.0	0.992	1.044	1.082	1.044	1.128	1.163	1.072
3.5	0.966	1.049	1.090	1.045	1.136	1.177	1.085
4.0	0.944	1.053	1.097	1.046	1.144	1.190	1.097

COMPOUND WEIRS.



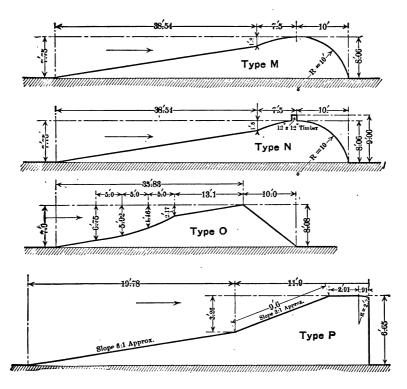
TRAPEZOIDAL WEIRS.





I.	Multi	ipliers of Disc	charge over S	II. Sharp-edged	Vertical Weir	of Same L	and p .
Head in Feet, h.	Туре А.	Type B.	Туре С.	Type D.	D with Vacuum.	Туре Е.	E with Vacuum.
0.5	0.968	1.060	1.043	1.069	1.088	1.069	1.069
1.0	1.071	1.079	1.040	1.079	1.106	1.079	1.079
1.5	1.077	1.091	1.037	1.084	1.117	1.088	1.092
2.0	1.081	1.096	1.027	1.057	1.092	1.063	1.083
2.5	1.077	1.093	1.015	1.041	1.079	1.049	1.081
3.0	1.074	1.090	1.005	1.028	1.068	1.039	1.080
3.5	1.071	1.087	0.996	1.018	1.059	1.029	1.079
4.0	1.069	1.085	0.989	1.009	1.051	1.021	1.078
		1	1	1	i	l	

COMPLEX WEIRS.



I. Head	II. Multipliers.								
in Feet, h.	Туре М.	Type N.	Туре О.	Туре Р.					
0.5	0.964	0.897	1.095	0.920					
1.0	0.965	0.946	1.088	0.915					
1.5	0.963	0.999	1.084	0.914					
2.0	0.949	1.025	1.069	0.935					
2.5	0.933	1.039	1.051	0.950					
3.0	0.920	1.052	1.035	0.962					
3.5	0.911	1.063	1.024	0.972					
4.0	0.903	1.072	1.014	0.982					

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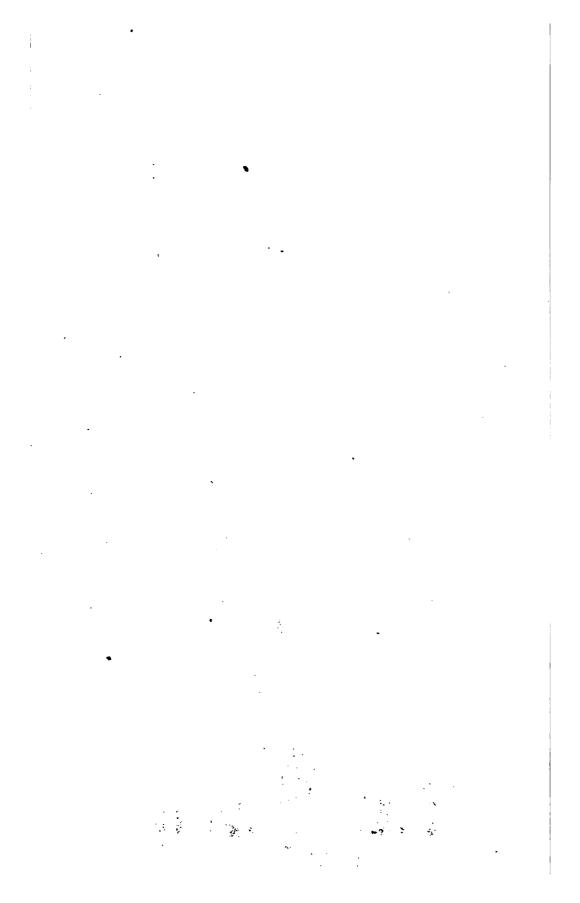
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